TREE PROTECTION GUIDELINES

All remaining trees are to have a tree protection zone (TPZ) established before commencement of any construction or delivery activities. The following guidelines are to be observed and practiced during all construction activities.

- Access is to be restricted into TPZ's with readily visible temporary tree fencing along the LOD which completely surrounds the protected areas of retained trees. Fences shall be constructed of chain link and be at least 4 ft tall, constructed using pier block, and
- major roots should be avoided while staking. • Highly visible signs spaced no further than 15 feet shall be placed along sides of the TPZ fencing.
- Construction materials or supplies, soil, debris, vehicles, and equipment are not to be parked or stored within TPZ.
- TPZ fences must be inspected prior to the beginning of any construction activities.
- Assess crew and contractor penalties, if necessary, to keep the TPZ's intact.
- Check the integrity of TPZ fences weekly, and repair or replace as
- Wood chips should be used if possible to spread above root zones within the TPZ's to a depth of 6-8 inches for temporary protection.
- Cement trucks must not deposit waste or rinse out trucks in the TPZ.

Avoid grade changes or trenching within or near the TPZ. If it is

- unavoidable, then follow the guidelines below. TPZ's may only be moved or accessed with permission from City Officials, and any work done within TPZ's must be done with a
- certified arborist present. • If roots need to pruned, they should be cut with pruning saws, made
- flush with the side of the trench.
- Trees should be watered twice a week if construction is to take place during hot summer months.

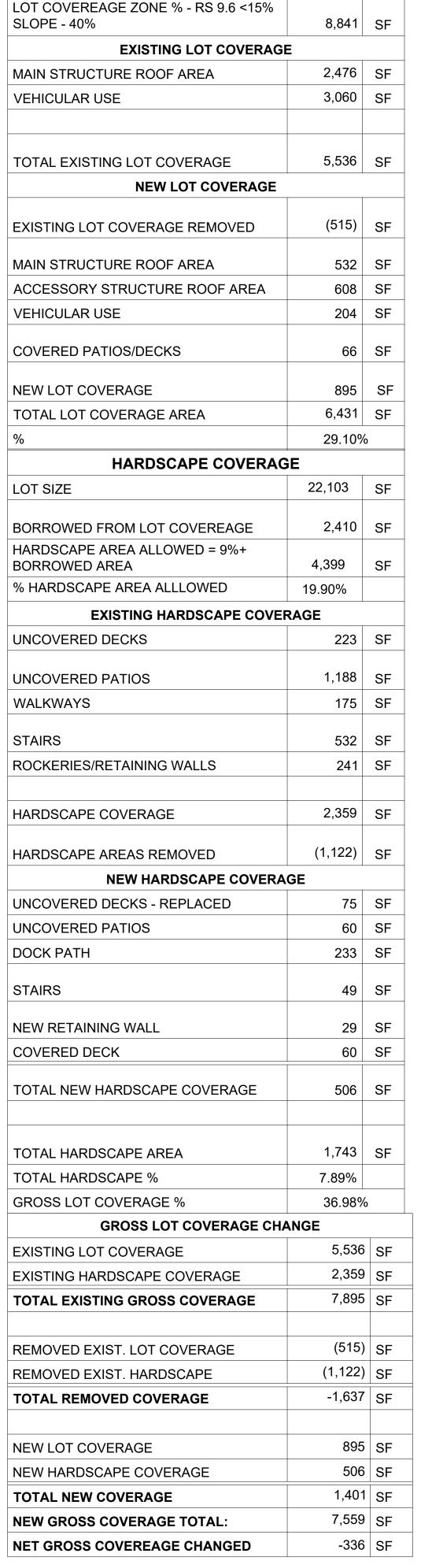
- If excavation occurs within the driplines of trees scheduled for retention, the following procedures must be followed to protect
- The contractor shall verify the vertical and horizontal location of existing utilities to avoid conflicts and maintain minimum clearances; adjustment shall be made to the grade of the new utility as required.

• The inner root zone shall not be disturbed or cut (inner root zone =

- half the drip line radius). ISA Certified arborist must work with equipment operators during trenching/ excavation. The Arborist should have a shovel, hand
- pruners, loppers, handsaw, and a sawsall. If roots one inch or larger are damaged by equipment, the Arborist shall stop the equipment and have the dirt excavated by hand until the root can be cleanly cut. A clean straight cut shall be made to remove the damaged portion of root, and if possible the roots should
- be covered in moist burlap until recovered with dirt the same day. Boring or tunneling under roots of existing trees is a viable alternative to trenching through roots. It shall be performed under the supervision of an ISA Certified Arborist, and no roots 1 inch in diameter or larger shall be cut.
- The grade shall not be elevated or reduced within the critical root zone of trees to be preserved without the Planning Official's authorization based on recommendations from a qualified professional. The Planning Official may allow coverage of up to one half of the area of the tree's critical root zone with light soils (no clay) to the minimum depth necessary to carry out grading or landscaping plans, if it will not imperil the survival of the tree. Aeration devices may be required to ensure the tree's survival.

	TREE RETENTION/ PROVIS	ION CALCULATION	ON			
EXISTING ON-SITE TRE	EES	RETAIN	REMOVE	DBH	CREDIT	
TREE #1	FLOWERING CHERRY	х		12"	2	
TREE #2	Cedrus Atlantica, Atlas Cedar	х		29.5"	10	
TREE #3	Quercus, Pin Oak	х		24.8"	8	
TREE #4	Acer palmatum, Japanese Maple	X		12.5"	2	
TREE #5	Acer palmatum, Japanese Maple		x	11.0"	1	
TREE #6	Cedrus Atlantica, Atlas Cedar	X		26.0"	9	
TREE #7	Acer Macrophyllum, Big Leaf Maple	X		17.4"	4	
TREE #8	Fraxines, Ash	X		21.0"	6	
TREE #9	Fraxines, Ash	Х		14.0"	3	
NEW TREES TO REPLA	ACE REMOVED					
TREE #10	Amelanchier Alnifolia, Western Serviceberry	X (NEW) REPLACEME NT		2"		
TREE #11	Amelanchier Alnifolia, Western Serviceberry	X (NEW) REPLACEME NT		2"		
EXISTING OFF-SITE TR	EES w/ OVERHANGING LIMBS					
TREE # OS 1	Cuppreseocyparis Leylandii, Layland	х		est 18"		
TREE # OS 2	Cuppreseocyparis Leylandii, Layland	х		est 21"		
TREE # OS 3	Populus Nigra, Black Cottonwood	Х		est 44"		
TOTAL CREDITS PROPOSED					45	
LOT SIZE			(0.50	ACRES	
TREES PER ACRE	PER KZC 95.33		:	30.0		
TOTAL CREDITS REQUIRED				1	5.0	

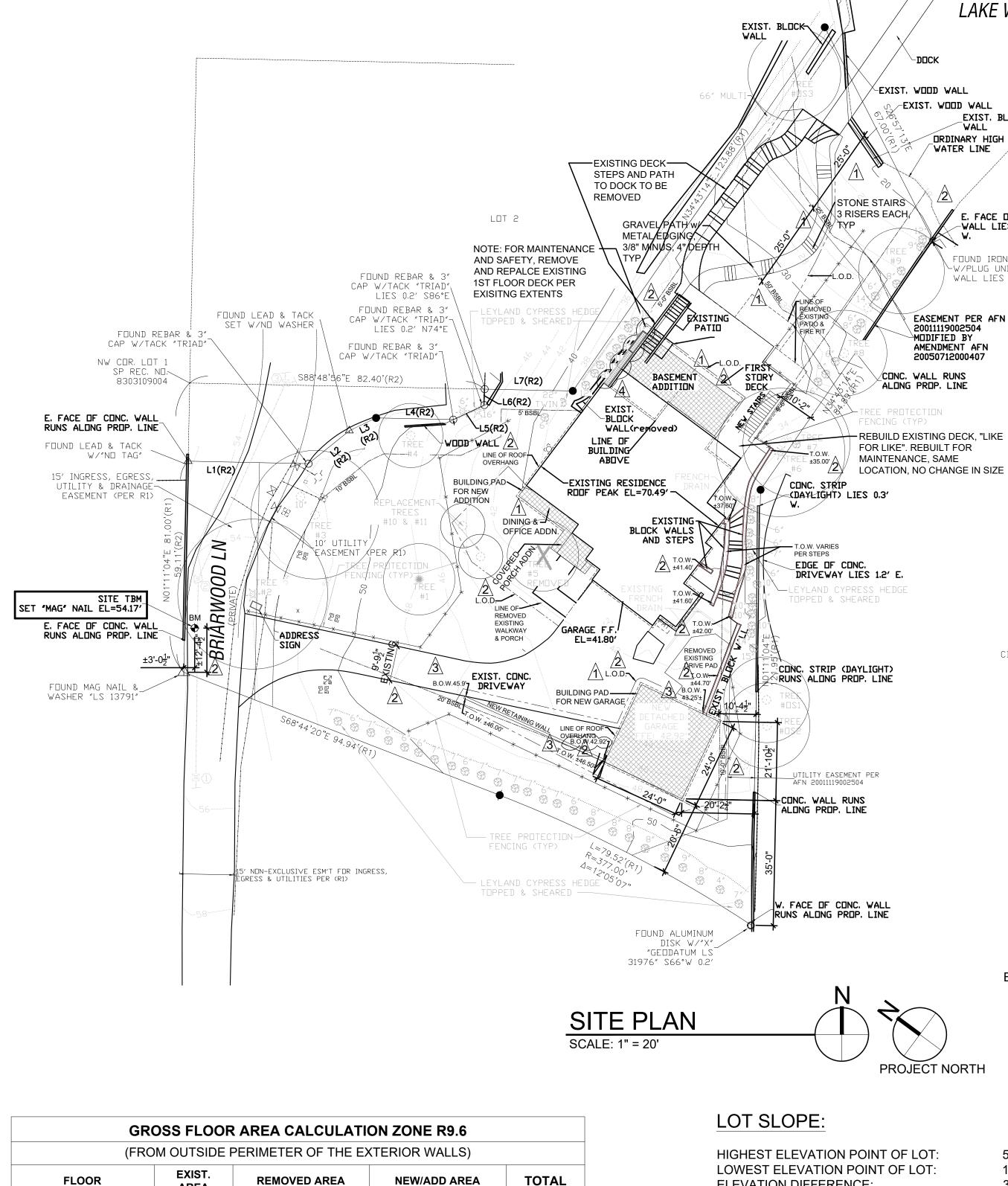
9820 SE 35TH PLACE, MERCER ISLAND, WA. 98040



LOT COVERAGE

LOT SIZE

22,103 SF



TOTAL 1,550 67 1,744 1,009 505 92 4,900

5,476

25.24%

FIRE A	REA CALC	ULATION	
(FROM INSIDE PER	IMETER OF T	HE EXTERIOR	R WALLS)
AREA			SQ. FTG.
BASEMENT			1,638
st FLOOR			1,663
2nd FLOOR			1,756
ATTACHED GRAGE			486
COVERED PORCH			64
COVERED DECKS			363
COVERED PATIO			126
TOTAL FIRE SF:			6,096

AREA

1,430

1,677

1,290

4,902

21,700 R9.6

PROPOSED GROSS FLOOR AREA PERCENTAGE:

PROPOSED GROSS FLOOR AREA SQUARE FOOTAGE;

679

ALLOWED GROSS | ALLOWED GROSS

FLOOR AREA (SF) | FLOOR AREA (%)

UPPER FLOOR

GROSS BASEMENT

STAIR CASE GFA

TOTAL BUILDING

ACCESSORY

BUILDING

LOT AREA

MAIN FLOOR

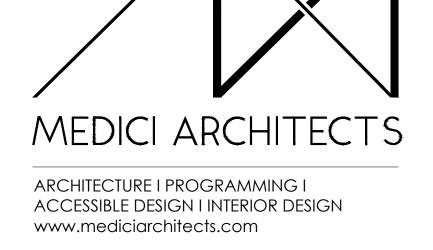
AREA

GARAGE

MODIFIER

HIGHEST ELEVATION POINT OF LOT: LOWEST ELEVATION POINT OF LOT: ELEVATION DIFFERENCE: HORIZONTAL DISTANCE BETWEEN POINTS: 254.7 FEET

LOT SLOPE: 14.3%



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REGISTRATION:

LAKE WASHINGTON

EXIST. BLOCK

E. FACE OF CONC.
WALL LIES 0.2'

-W/PLUG UNDER ROCK

WALL LIES 0.3' N60°W

LOT 1

CITY OF MERCER ISLAND L.L.A

REC. NO. 20021125900027 APN 0824059027

22,103±S.F./0.51±ACRES

NO EXCAVATION

BEYOND FOUNDATIONS

CALL 48 HOURS BEFORE YOU DIG

811 OR 1-800-424-5555

54.4 FEET

18.0 FEET

36.4 FEET

SSMH SANITARY SEWER MANHOLE

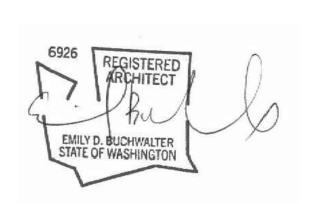
S 88° 52' 32" E 296.84' BEARING

ORDINARY HIGH

EASEMENT PER AFN

^{_}M□DIFIED BY

AMENDMENT AFN



INTAKE: DATE: **REVISIONS:** . Robin Proebsting comments 12-04-20

2./2\PER COMMENT 01-2005-081-SUB1-PLANS 04-01-2021 3./3\ADDED NOTES PER 2005-081-SUB2-PLANS 05-20-2021 4. A POST PERMIT REVISIONS 01-27-2022

PROJECT / CLIENT:

9820 SE 35TH PLACE ACHIN & MARY CHEN 9820 SE 35TH PLACE MERCER ISLAND, WA 98040

JOB ADDRESS: 9820 SE 35TH PLACE MERCER ISLAND, WA 98040

PARCEL # 082405-9027

DRAWING NAME:

SITE PLAN

Drawn Bv: JMG.RB Checked By: EB Owner Approval:

CONSTRUCTION DOCUMENTS

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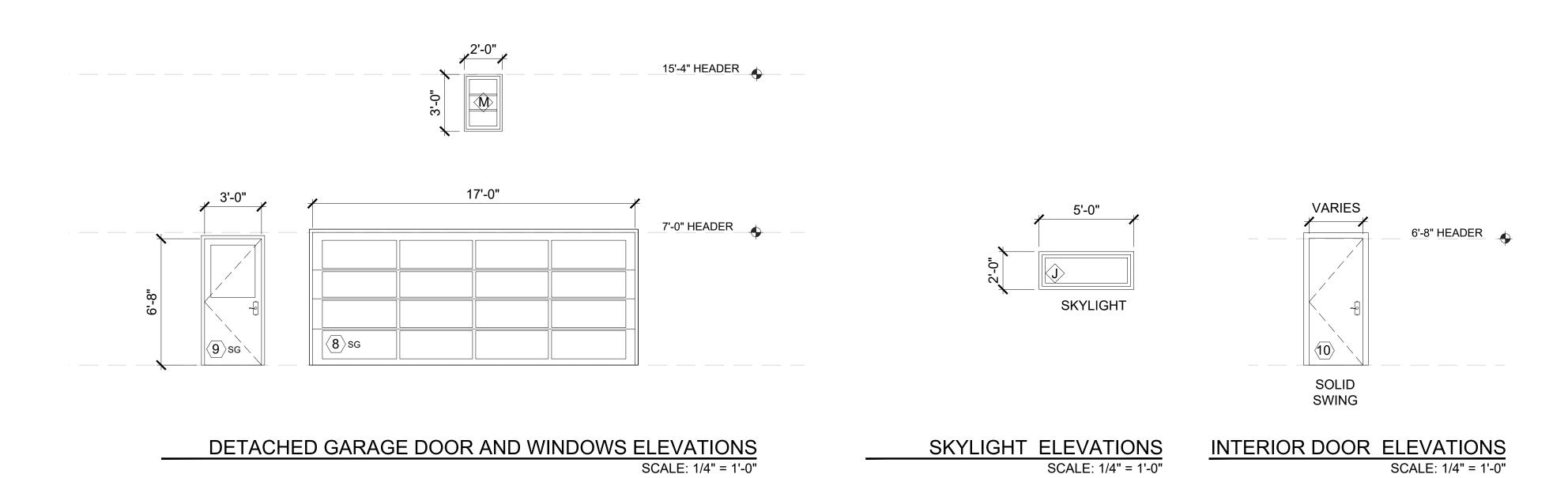
PROJECT No.: 2020 007

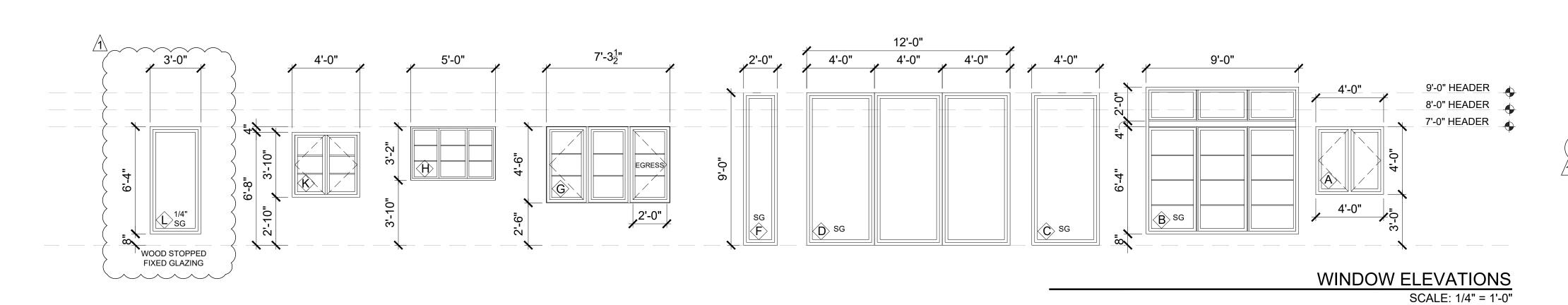
12-22-2020

DATE:

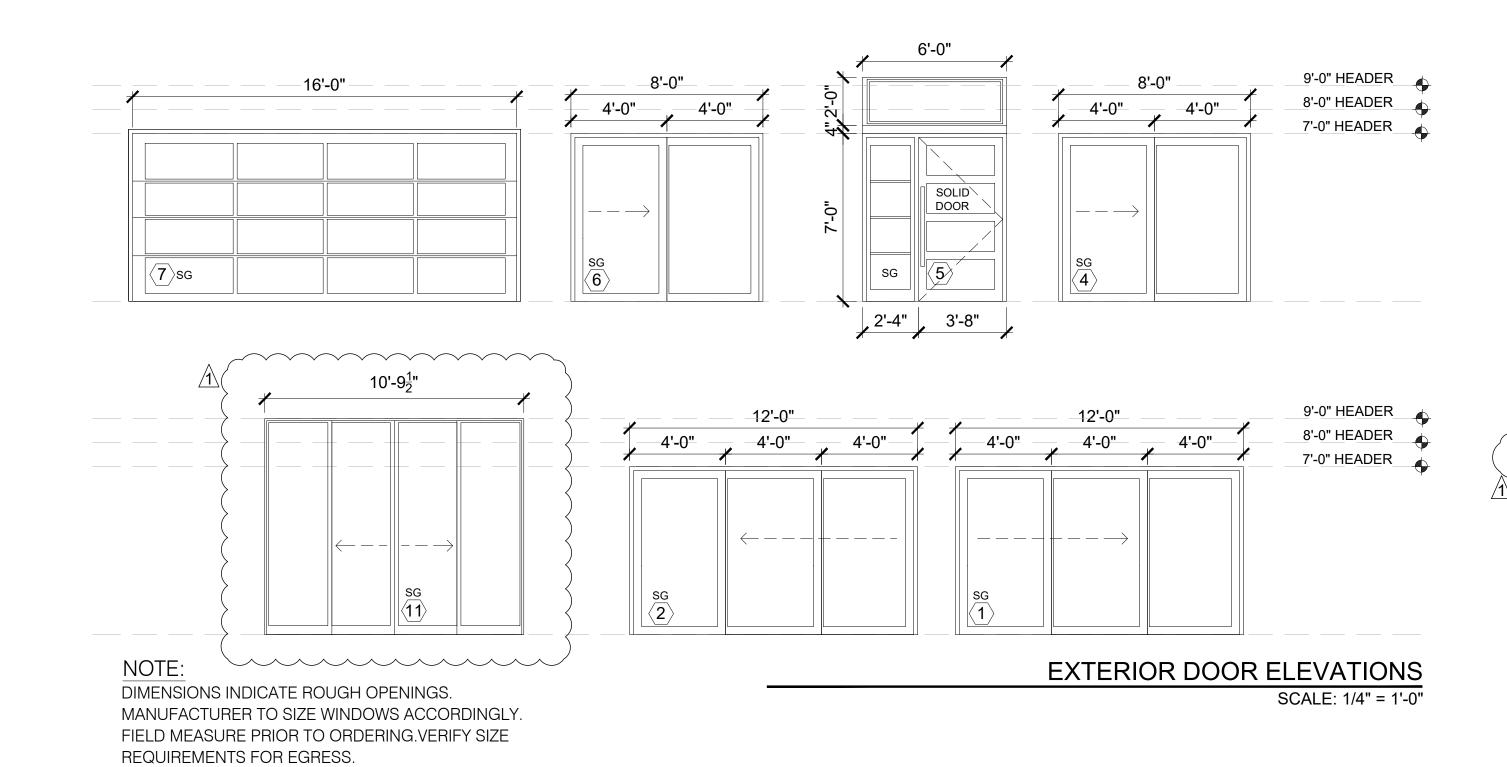
PLOT SCALE: 1:1

SYMBOL LEGEND: _ CENTER LINE FG 386.0' FINISHED GRADE SANITARY SEWER EG 386.0' EXISTING GRADE **GAS SERVICE** _ STORM DRAINAGE TOW 386.0' TOP OF WALL _ UNDERGROUND POWER __w___ WATER SERVICE BOW 386.0' BOTTOM OF WALL -----LINE OF ROOF ABOVE ---- ABE RECTANGLE B-411.10' b-34.25' ABE RECTANGLE MIDPOINT BUILDING FOOTPRINT - · - · - PROPERTY LINE TREES - SETBACK LINE .. EXISTING CONTOUR _ PROPOSED CONTOUR TREE DRIPLINE ---- EASEMENT LINE TREE FENCE TREE TO BE REMOVED RETAINING WALL LINE OF DISTURBANCE POWER POLE WITH LIGHT CONCRETE BM BENCH MARK





SCALE: 1/4" = 1'-0"



SCALE: 1/4" = 1'-0"

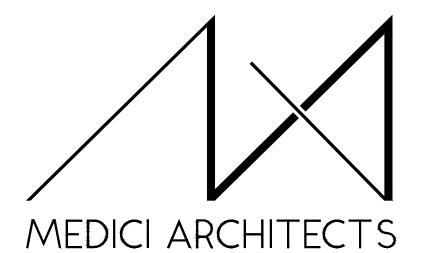
NO	Qty.	LOCATION	W	Н	MANUF	TYPE	HARDWARE	REMARKS
10	1	OFFICE	2'-8"	6'-8"	TBD	SOLID SWING		
							ALL HARDWARE TO BE	
							BRUSHED NICKEL FINISH 2- PAIR OF BUTT HINGES FOR	
							8'-0" DOORS	
	1	DOOR COUNT						
NOTES:			ı				'	
. GENERA	AL CONTR	ACTOR SHALL PROVIDE MANUFACTURE	ER'S DATA ON ALL	WINDOWS	SHOWING C	OMPLIANCE WITH THE 2015 W	/ASHINGTON STATE ENERGY CODE.	
 2. ALL EXT	ERIOR TR	UE DIVIDED FIXED TRANSOM GLAZING	TO BE POSITIONE	D AT UPPE	R SASH LOC	ATION.		
		R TYPES & HARDWARE W/OWNER PRIOR						

					WINDC	W SCH	IEDUL	.E			
NO	Qty.	LOCATION	WIDTH	HEIGHT	AREA	MANUF.	U-VAL	TYPE	SCREEN	HARDWARE	REMARKS
Α	1	NEW EXERCISE ROOM	4'-0"	4'-0"	16.00	TBD	0.28	CSMT/CSMT	Y	TBD	
В	1	FORMAL DINING ROOM	9'-0"	8'-4"	74.70	TBD	0.28	FIXED	N	TBD	TRANSOM,GRIDS
С	1	FAMILY ROOM	4'-0"	9'-0"	36.00	TBD	0.28	FIXED	N	TBD	SAFETY GLASS
D	1	FAMILY ROOM	12'-0"	9'-0"	18.00	TBD	0.28	FIXED	N	TBD	SAFETY GLASS, MULLED
Е	1	EXISTING WINDOWS									
F	1	FAMILY ROOM	2'-0"	9'-0"	18.00	TBD	0.28	FIXED	N	TBD	SAFETY GLASS
G	1	BEDROOM 3	7'-3.5"	4'-6"	32.85	TBD	0.28	CSMT/CSMT/CSMT	Υ	TBD	EGRESS, GRIDS
Н	1	ABOVE FOYER	5'-0"	3'-2"	15.50	TBD	0.28	FIXED	N	TBD	GRIDS, SAFETY GLASS
I	1	NOT USED	0	0	0.00						
J	1	FOYER	2'-0"	5'-0"	10.00	TBD	0.43	SKYLIGHT	N	TBD	SKYLIGHT
√K √	1	NEW OFFICE	4'-0"	3'-10"	8.34	TBD	0.28	CSMT/CSMT	Y	TBD	GRIDS
L	2	NEW EXERCISE RM INTERIOR	3'-0"	6'-4"	37.80	TBD		FIXED	N	TBD	SAFETY G;ASS
<u></u>	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	DETACHED GARAGE WINDOW	2-0"	3,-0,	6.00	TBD		†HXÉD †		TÊD	^\non`conditioned^
					000.00		0.28		64.23		
					229.39		0.28				
					10.00	SF	0.43		4.3		NOTE: SEE A0.3 & A4.0,1,2 FOR WINDOW DIVISIONS
	14	WINDOW COUNT						U X A =	68.53		
NOTES:											

2. ALL EXTERIOR TRUE DIVIDED FIXED TRANSOM GLAZING TO BE POSITIONED AT UPPER SASH

3. ALL WINDOWS TO BE NFRC CERTIFIED AND LABELED

NO	Qty.	LOCATION	W	Н	MANUF.	AREA	U-VAL	TYPE	CONFIGURATION	HARDWARE	REMARKS
1	1	NEW EXERCISE ROOM	12'-0"	7'-0"	TBD	84.0	0.28	SLIDING	XXO		SAFETY GLASS
2	1	NEW SITTING ROOM	12'-0"	7'-0"	TBD	84.0	0.28	SLIDING	XXO		SAFETY GLASS
3	0	NOT USED	0	0	TBD	0.0	0.28				
4	1	KITCHEN	8'-0"	7'-0"	TBD	56.0	0.28	SLIDING	ХО		SAFETY GLASS
5	1	FOYER	6'-0"	9'-0"	TBD	54.0	0.28	SOLID SWING WITH SIDE LITE & TRAMSOM		ALL HARDWARE TO BE BRUSHED NICKEL FINISH 2- PAIR OF BUTT HINGES FOR 6'-8"/ 8'-0" DOORS	SAFETY GLASS
6	1	MASTER BEDROOM	8'-0"	7'-0"	TBD	56.0	0.28	SLIDING	XO		SAFETY GLASS
7	1	GARAGE	16'-0"	7'-0"	TBD			OVER HEAD GARAGE DOOR	Х	GARAGE DOOR OPENER, S PATT	
8	1	DETACHED GARAGE	17'-0"	7'-0"	TBD			OVER HEAD GARAGE DOOR	Х	GARAGE DOOR OPENER, S PATT	
9	1	DETACHED GARAGE ENTRY	3'-0"	8'-0"	TBD		0.28	SOLID SWING	X	ALL HARDWARE TO BE BRUSHED NICKEL FINISH 2- PAIR OF BUTT HINGES FOR 6'-8"/ 8'-0" DOORS	1/2 LIGHT, SAFTY GLASS
11	1	LIVING ROOM	11'-0"	9'-0"	TBD	99.0	0.28	SLIDING	OXXO		SAFETY GLASS
TAL^					\\SE\\\	433.0	0.28	TOTALUXA	121.2		
TES:											
SENERA	L CONTRA	CTOR SHALL PROVIDE MANU	FACTURER'S	DATA ON A	LL WINDOWS S	HOWING COM	⊥ PLIANCE WI7	□ TH THE 2015 WASHINGTON	STATE ENERGY CODE.		
ALL EXT	ERIOR TRI	JE DIVIDED FIXED TRANSOM	GLAZING TO E	BE POSITION	NED AT UPPER	SASH LOCATION	 ON.				
		TYPES & HARDWARE W/OWN									
		G UN-HEATED FROM HEATED				 015					
		LAZING TO BE NFRC CERTIFIE									



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REVISIONS:	DATE:
1. POST PERMIT REVISION	01-11-2022
2.	
3.	
4.	
5	

PROJECT / CLIENT: **9820 SE 35TH PLACE**

ACHIN & MARY CHEN 9820 SE 35TH PLACE MERCER ISLAND, WA 98040

JOB ADDRESS: 9820 SE 35TH PLACE MERCER ISLAND, WA 98040 PARCEL # 082405-9027

WING NAME:

HEDULES

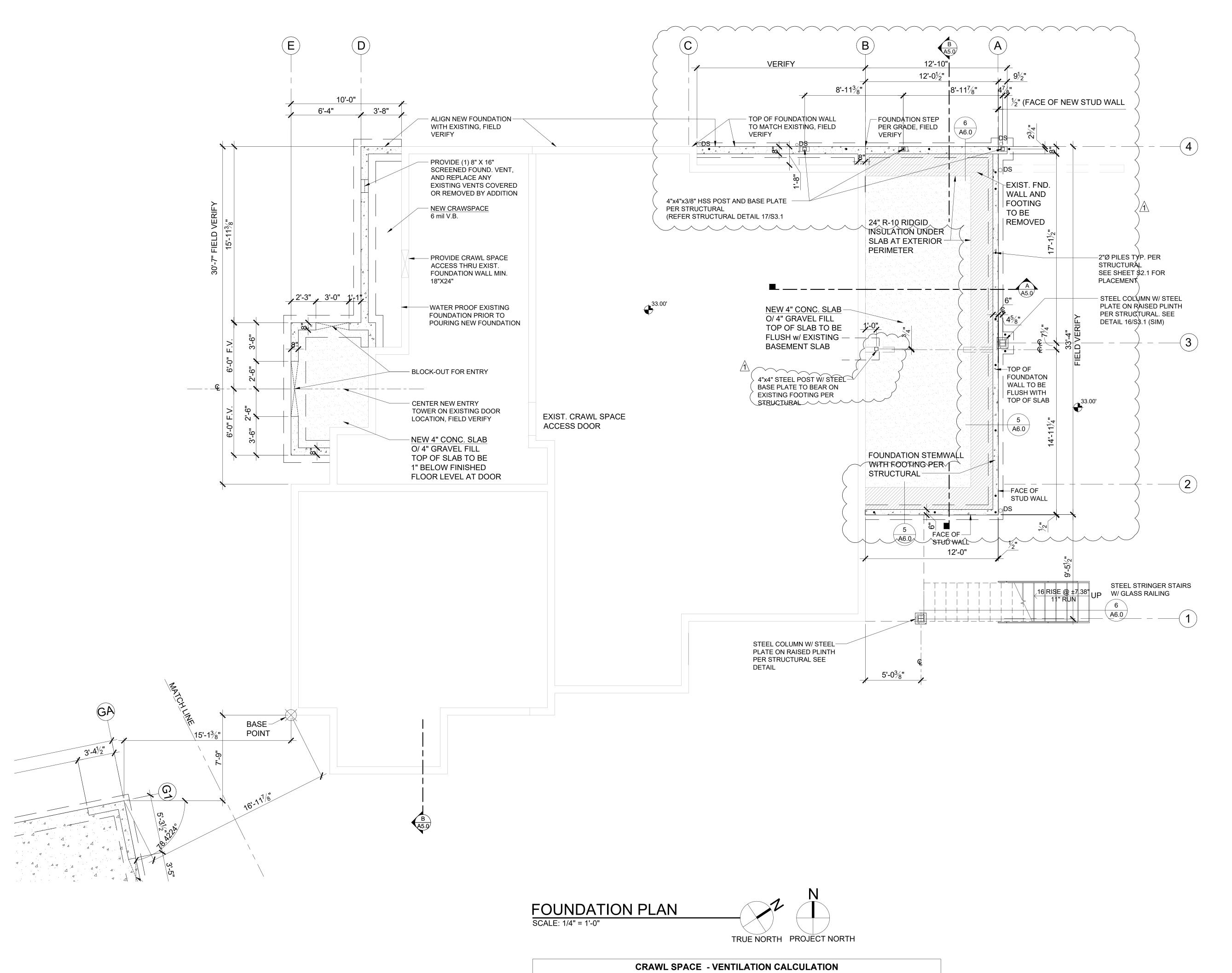
vn By: JMG,RB cked By: EB er Approval:

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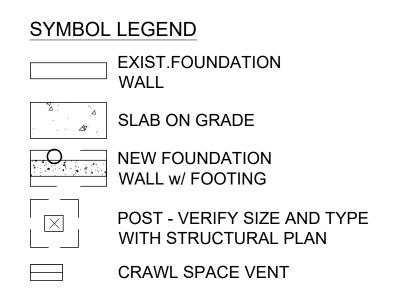
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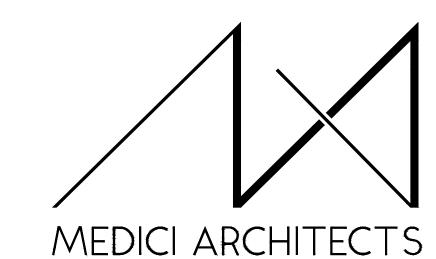
PROJECT No.: 2020 007 DATE: 12-22-2020



Ci	RAWL SPACE -	VENTILATION CALCULAT	ION	
Added Crawl Space Area:	51	s.f.		
Ventilation Required:	51	s.f. x 144 s.i.1 /1,500*=	4.9	s.i. Req'd
Use:	16"x8"	Foundation Vents		
Vent Area =	98.0	s.l 25% reduction + 1/4" mesh	73.5	s.l.
Number of vents required:	4.9	s.l. / vent area	0.1	vents
Provide:	1.0		73.5	Provided
Total Min. Ventilation Provided =	73.5	s.i. IS GREATER THAN	4.9	s.i. Req'd

* 2015 IRC - PER R408.1 THE TOTAL AREA OF VENTILATION OPENINGS SHALL BE PERMITTED TO BE REDUCED TO 1/1,500 OF THE UNDER-FLOOR AREA WHERE THE GROUND SURFACE IS COVERED WITH AN APPROVED CLASS I VAPOR RETARDER MATERIAL AND THE REQUIRED OPENINGS ARE PLACED TO PROVIDE CROSS VENTILATION OF THE SPACE



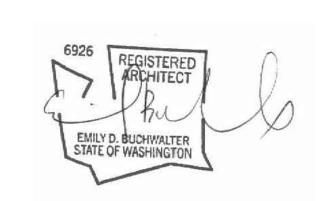


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> **9820 SE 35TH PLACE** ACHIN & MARY CHEN 9820 SE 35TH PLACE MERCER ISLAND, WA 98040

PROJECT / CLIENT:

JOB ADDRESS: 9820 SE 35TH PLACE MERCER ISLAND, WA 98040 PARCEL # 082405-9027

DRAWING NAME:

FOUNDATION PLAN

Drawn By: JMG,RB Checked By: EB Owner Approval:

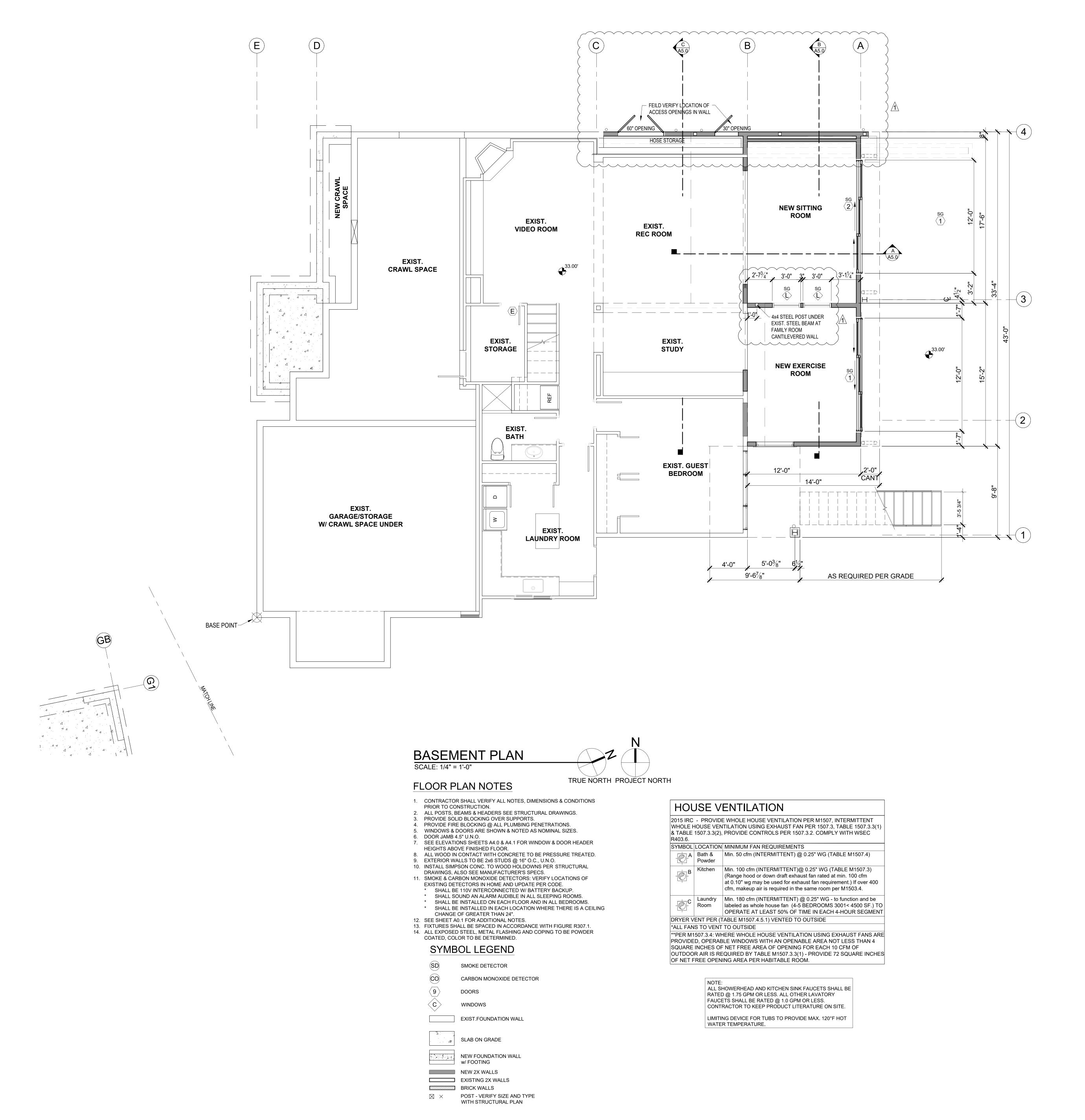
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PROJECT No.: 2020 007 DATE: 12-22-2020

PLOT SCALE: 1:1



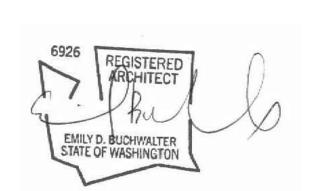
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INTAKE: DATE: **REVISIONS:** 1. POST PERMIT REVISIONS 01-24-2022

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PROJECT / CLIENT:

JOB ADDRESS: 9820 SE 35TH PLACE MERCER ISLAND, WA 98040 PARCEL # 082405-9027

DRAWING NAME:

BASEMENT CONSTRUCTION PLAN

Drawn By: JMG,RB Checked By: EB Owner Approval:

CONSTRUCTION DOCUMENTS

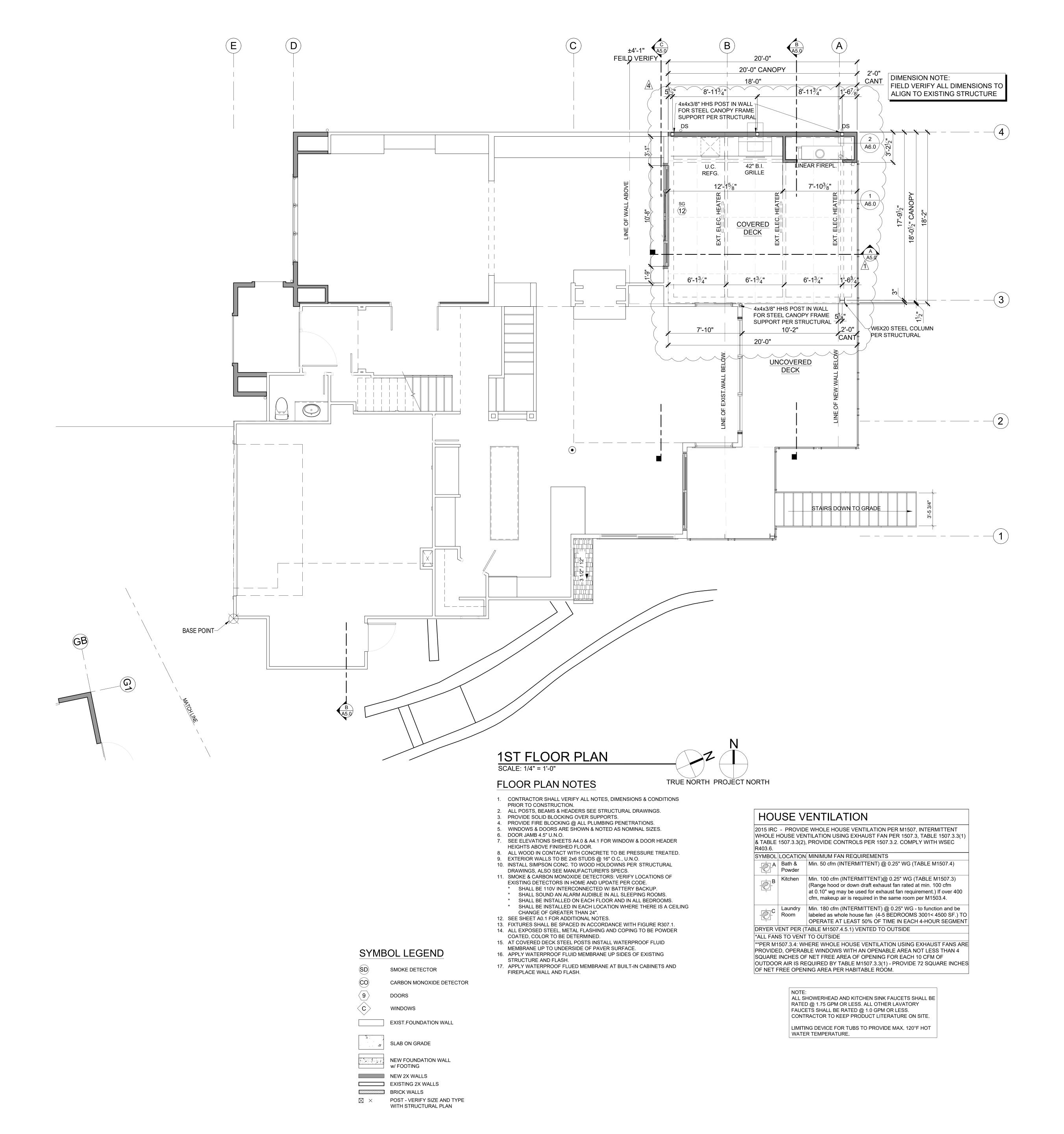
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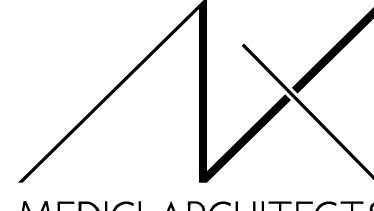
APPROVED FOR CONSTRUCTION:

PROJECT No.: 2020 007

DATE: 12-22-2020

PLOT SCALE: 1:1



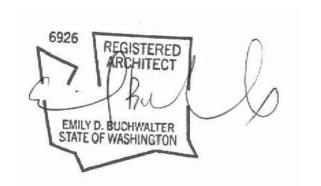


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DATE: DATE: **REVISIONS:** 1./1\ COMMENT 01-2005-081-SUB1-PLANS 04-01-2021 2./2\ UPDATED COMMENT PER SUB2-PLANS 06-03-2021 3./3\ UPDATED COMMENT PER SUB3-PLANS 07-13-2021

4./4\ UPDATED DOOR OUTDOOR KITCHEN 01-28-2022

PROJECT / CLIENT:

9820 SE 35TH PLACE

ACHIN & MARY CHEN 9820 SE 35TH PLACE MERCER ISLAND, WA 98040

JOB ADDRESS:

9820 SE 35TH PLACE MERCER ISLAND, WA 98040 PARCEL # 082405-9027

DRAWING NAME:

1ST FLOOR CONSTRUCTION PLAN

Drawn By: JMG,RB Checked By: EB Owner Approval:

PHASE:

CONSTRUCTION DOCUMENTS

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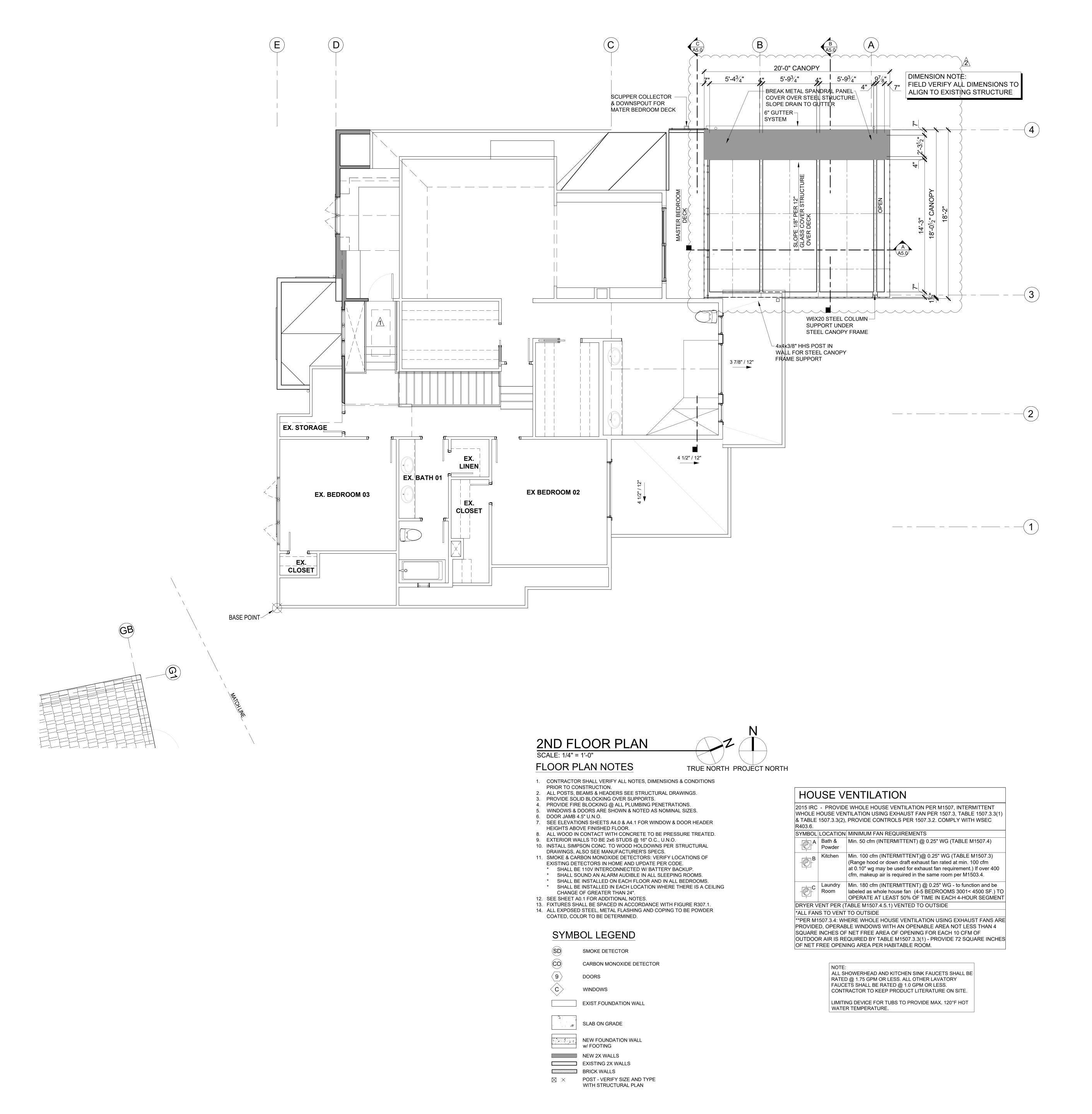
APPROVED FOR CONSTRUCTION:

12-22-2020

PROJECT No.: 2020 007

DATE:

PLOT SCALE: 1:1

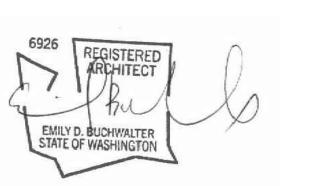


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PROJECT / CLIENT:

9820 SE 35TH PLACE ACHIN & MARY CHEN

ACHIN & MARY CHEN 9820 SE 35TH PLACE MERCER ISLAND, WA 98040

JOB ADDRESS:

9820 SE 35TH PLACE MERCER ISLAND, WA 98040 *PARCEL* # 082405-9027

DRAWING NAME:

2ND FLOOR CONSTRUCTION PLAN

Drawn By: JMG,RB
Checked By: EB
Owner Approval:

PHASE:

CONSTRUCTION DOCUMENTS

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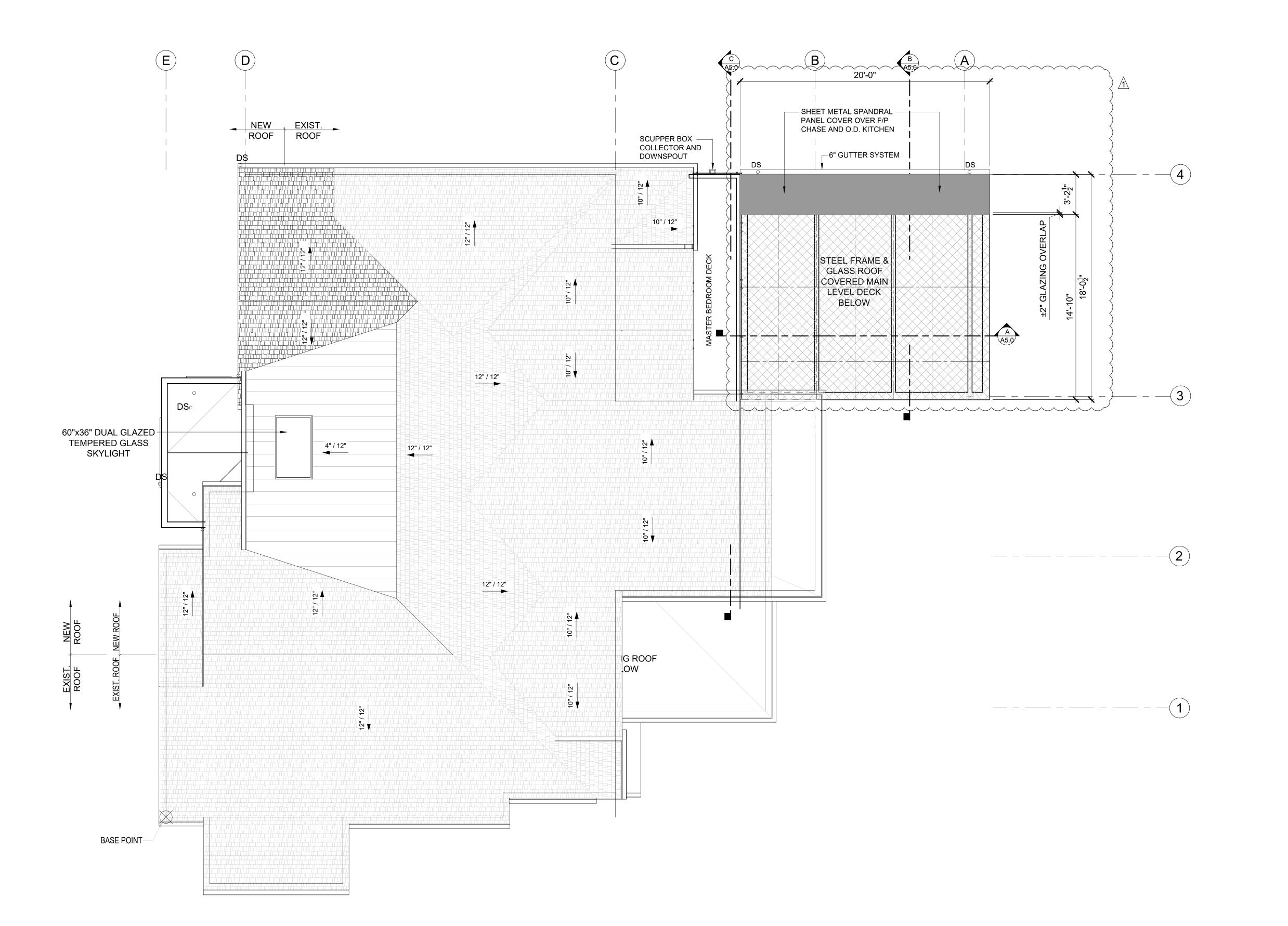
APPROVED FOR CONSTRUCTION:

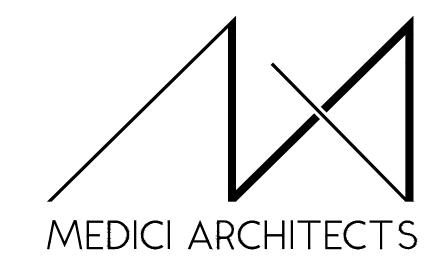
PROJECT No.: 2020 007

DATE:

PLOT SCALE: 1:1

12-22-2020

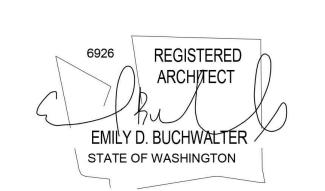




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REGISTRATION:



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REVISIONS:	DATE:
1/1 POST PERMIT UPDATES	01-28-2022
2.	
3.	
4.	
5.	

9820 SE 35TH PLACE ACHIN & MARY CHEN

ACHIN & MARY CHEN 9820 SE 35TH PLACE MERCER ISLAND, WA 98040

JOB ADDRESS:

9820 SE 35TH PLACE MERCER ISLAND, WA 98040 *PARCEL # 082405-9027*

ROOF PLAN
SCALE: 1/4" = 1'-0"

Stick built Roof Construction:

SmartVent Shingle vent (upper or ridge)

SmartVent Shingle vent (lower roof edge)

Roof Area:

Provide:

Ventilation Required:

Proposed Ventilation:

Total Ventilation Provided

ROOF - VENTILATION CALCULATION

356.7 s.f.

356.7 s.f. x 144 s.i. / 300*=

20.0 I.f. Upper Ventilation =

180.0 s.i. IS GREATER THAN

20.0 I.f. Eave Edge Ventilation =

4.5 s.i. nfa / l.f.=

4.5 s.i. nfa / l.f. =



171.2 s.i. Req'd

4.5 s.i. / l.f.

171.2 s.i. Req'd

4.5 s.i.

90.0

90.0

TRUE NORTH PROJECT NORTH

ROOF PLAN

DRAWING NAME:

Drawn By: JMG,RB

Owner Approval:
PHASE:

PHASE:

Checked By: EB

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APPROVED FOR CONSTRUCTION:

PROJECT No.: 2020 007

DATE: 12-22-2020

PLOT SCALE: 1:1

A3.0

ELEVATIONS NOTES & KEY NOTES:

- 1. VERIFY SHEAR WALL NAILING & HOLDOWNS PER STRUCTURAL PLAN & SCHEDULE PRIOR TO INSTALLING SIDING.
- 2. MATCH EXISTING CEDAR SIDING PROFILE AND EXPOSURE, PAINT TO MATCH. INTERWEAVE NEW CEDAR SIDING TO OLD AND EXTEND VAPOR BARRIER MINIMUM OF 6 INCHES. TRANSITIONS TO BE SEAMLESS.
- 3. CAULK ALL EXTERIOR JOINTS & PENETRATIONS.

PENETRATIONS PER I.R.C. R903.2 & R903.2.1.

- 4. AT NEW AND REMODELED CONSTRUCTION AREAS PROVIDE APPROVED CORROSION
- RESISTANT FLASHING AT EXTERIOR WALL ENVELOPE PER I.R.C. R703.4. 5. AT NEW AND REMODELED CONSTRUCTION AREAS PROVIDE FLASHING AT ROOF
- 6. AT NEW AND REMODELED CONSTRUCTION AREAS PROVIDE PRE FINISHED CONTINUOUS ALUMINUM GUTTERS, SCUPPER AND DOWN SPOUTS - COLOR TO MATCH ADJACENT EXTERIOR MATERIAL FINISH. ROOF DRAINS AND SCUPPERS SHALL BE INSTALLED PER IRC SECTION R903.4. PROVIDE EMERGENCY OVERFLOW PER IRC SECTION 1503.4.1. TYPICAL SEE ROOF PLAN SHEET A3.0. DIRECT CONNECT FOOTING DRAINS AND DOWN SPOUTS PER CIVIL DRAWINGS.
- 7. SEE SHEET A0.1 FOR ADDITIONAL NOTES.
- 8. STUCCO VENEER: 3-COAT PORTLAND CEMENT STUCCO SHALL HAVE A SCRATCH, BROWN AND FINISH COATS OF PORTLAND CEMENT EXTERIOR PLASTER PER IRC SECTION R703.6.2; SAND FINISH COAT WITH INTEGRAL COLOR, OVER EXTERIOR METAL LATH PER IRC SECTION R703.6.1. TOTAL THICKNESS APPROXIMATELY 7/8". PROVIDE WEEP SCREEDS PER IRC SECTION R703.6.2.1
- 9. LIGHTING AT EXTERIOR DOORS, TYP.
- 10. POWDER COATED COPING
- 11. TPO ROOFING: MECHANICALLY ATTACHED ROOFING SYSTEM, LIGHT GRAY, SCRIM-REINFORCED THERMOPLASTIC POLYOLEFIN (TPO) MEMBRANE. PERIMETER SHEETS ARE INSTALLED ALONG THE BUILDING EDGES AND FIELD MEMBRANE SHEETS ARE MECHANICALLY ATTACHED TO THE ROOF DECK WITH THE APPROPRIATE FASTENERS AND FASTENING PLATES. ADJOINING SHEETS OF MEMBRANE ARE OVERLAPPED AND JOINED TOGETHER WITH A MINIMUM 1-1/2" WIDE HOT AIR WELD. INSTALL PER MANUFACTURER
- 12. MATCH EXISTING SHAKE ROOF AND FINISH TO MATCH, INSTALL PER INDUSTRY STANDARDS
- 13.12" STANDING SEAM METAL ROOFING, INSTALL PER INDUSTRY STANDARDS. COLOR FINISH TO BE SELECTED BY OWNER.
- 14.POWDER COATED STEEL FRAME FOR INSTALLATION OF TEMPERED LAMINATED GLASS
- ROOF CANOPY, INSTALL BY ROOF CANOPY MANUFACTURER. 15. POWDER COATED STEEL COLUMN OR C-CHANNEL PER STRUCTURAL.
- 16. FIRE PLACE VENT
- 17. HOOD VENT OVER BARBEQUE.
- 18. FRONT ENTRY DOOR: FRONT ENTRY DOOR SHALL BE ALUMINUM, MINIMUM1-3/4" THICK, 42" WIDE SINGLE SOLID DOOR WITH ONE SIDELIGHT AND TRANSOM WINDOW ABOVE, DOUBLE-GLAZED SAFETY GLASS, WITH LOW-E. PROVIDE ANODIZED METAL THRESHOLD, CYLINDER ENTRY LOCK ACCESS AND DEADBOLT DRILLING. U-VALUE OF DOORS TO BE 0.30 (2015 WSEC) OR BETTER. PROVIDE EUTHERM ALUMINUM DOOR OR EQUAL AS APPROVED BY ARCHITECT.

INCHES IN DIAMETER.

TO THE STRUCTURE.

PER IRC - 301.5 CONCENTRATED LOAD. HANDRAILS AND GUARDS SHALL BE ABLE TO RESIST A SINGLE

CONCENTRATED LOAD OF 200 POUNDS, APPLIED IN

ANY DIRECTION AT ANY POINT ALONG THE TOP, AND TO TRANSFER THIS LOAD THROUGH THE SUPPORTS

R312.2.1 - WINDOW SILLS. IN DWELLING UNITS, WHERE

MORE THAN 72" ABOVE THE FINISHED GRADE OR SURFACE BELOW, THE LOWEST PART OF THE CLEAR

THE OPENING OF AN OPERABLE WINDOW IS LOCATED

OPENING OF THE WINDOW SHALL BE A MINIMUM OF 24

ABOVE THE FINISHED FLOOR OF THE ROOM IN WHICH THE WINDOW IS LOCATED. OPERABLE SECTION OF

WINDOWS SHALL NOT PERMIT OPENINGS THAT ALLOW

OPENINGS ARE LOCATED WITHIN 24" OF THE FINISHED

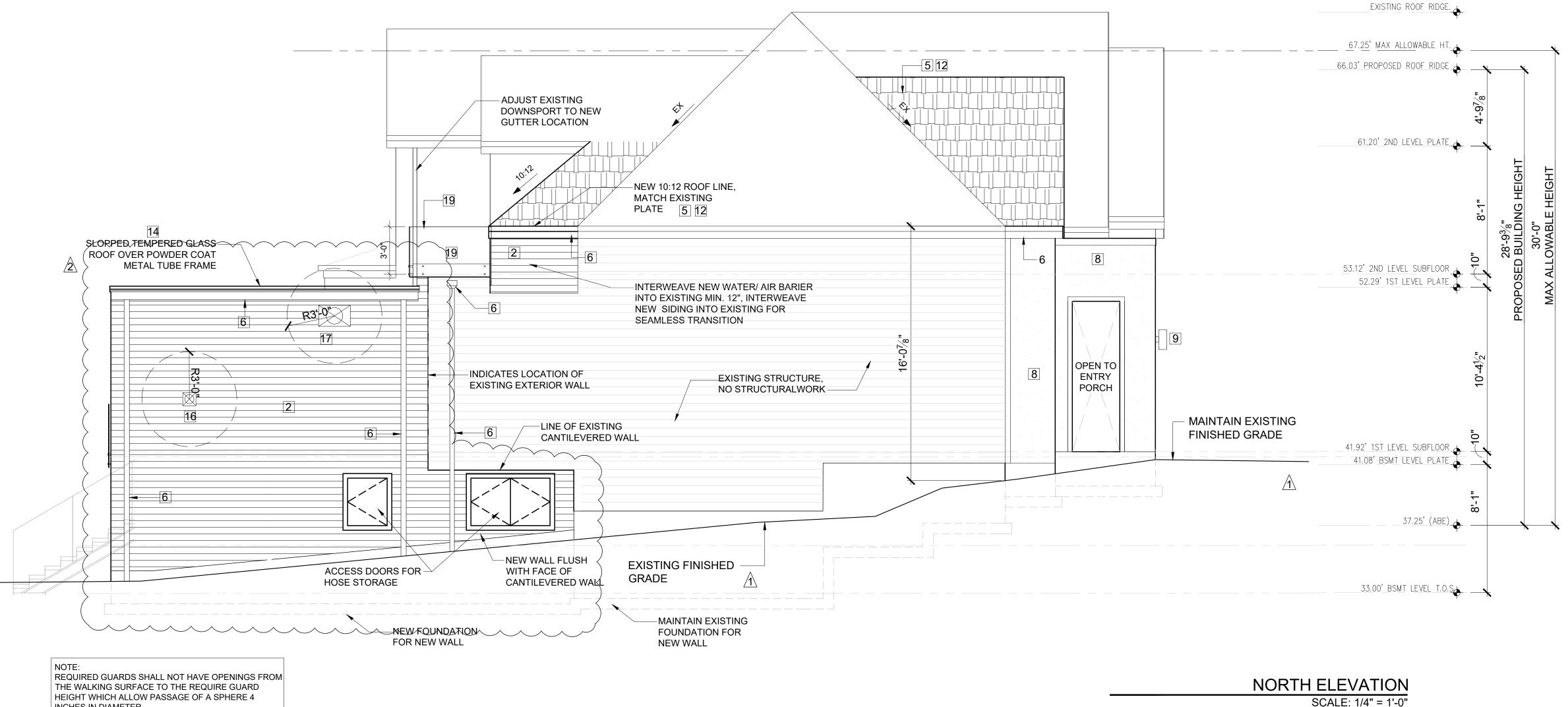
EXISTING FINISHED -GRADES

PASSAGE OF A 4" DIAMETER SPHERE WHERE SUCH

- 19. GLASS RAILING HANDRAIL: SIDE MOUNTED FRAMELESS GLASS RAIL SYSTEM WITH NON-GLARE TEMPERED GLASS PANELS.
- 20. WINDOWS: (CLIMATE ZONE 4C OF THE 2015 WSEC TABLE R402.1.1) ALL WINDOWS SHALL BE DOUBLE-PANED MINIMUM, PERFORMANCE AND CONSTRUCTION TO CONFORM WITH IRC SECTION R612. HARDWARE FINISH SHALL MATCH DOOR HARDWARE. ALL CASEMENT OPENINGS SHALL HAVE ROTO HARDWARE. ALL OPENINGS WEATHER-STRIPPED BY MANUFACTURER; GENERAL CONTRACTOR SHALL INSTALL "Z"-FLASHING AT HEADS OF ALL WINDOWS AND SEAL WINDOW PERIMETER PER MANUFACTURER'S SPECIFICATIONS.
- 21.PORCELAIN PAVERS DECK SYSTEM INSTALL PER PORCELANOSA MANUFACTURER INSTRUCTIONS.
- 22.WOOD DECKING OVER RIPPED CEDAR DECK JOIST OVER 3/4" MARINE BOARD WATER MEMBRANE ROOFING PER IRC. R905.13 AND CLOSED CELL SPAY FOAM R-38. 23.STONE TILE VENEER.

..\..\..\..\01 GENERAL\01.08 PERMIT FILES\jpg FILES\Dutdoor kitchen 3D.png

24.BLACKEN STAINLESS STEEL



_ 67.25' MAX ALLOWABLE HT. 66.036(PROPOSED ROOF RIDGE + + REPLACE EX WDW WITH NEW 61.20' 2ND LEVEL PLATE DOOR, RETAIN EX. HDR. -NEW 10: 12 SHAKE PER ROOF PLAN, FIELD VERIFY PITCH 5 12 FRAME NEW PONYWALL TO MATCH EXISTING ADJACENT HEIGHT. DRAWING NAME: 53.12' 2ND LEVEL SUBFLOOR 1/8":12 52.29' 1ST LEVEL PLAT -GLASS CANOPY ROOF BIYHQIRIS, SLOPE 1/8":12 TOWARDIMETAL GUTTER MATCH EXISTING HOZONTOADOAR SIDING PROFILE & REVEANDANI CORNER BOARD SIZE SEE SITE PLAN&FOR DOWNSPOUT LOCATIONS 41.92' 1ST LEVEL SUBFLOOR 41.08' BSMT LEVEL PLATE 37.25' (ABE) 🔶 ——— \leftarrow - -33.00' BSMT LEVEL T.O.S. - EXISTING FINISHED

EAST ELEVATION

SCALE: 1/4" = 1'-0"

EXISTING ROOF RIDGE.

MEDICI ARCHITECTS

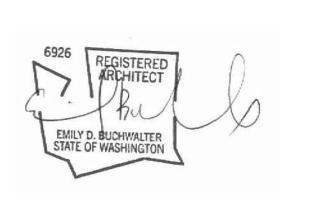
ARCHITECTURE I PROGRAMMING I

ACCESSIBLE DESIGN | INTERIOR DESIGN

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www.mediciarchitects.com

REGISTRATION:



DATE: **INTAKE: REVISIONS:** 1./\ PER COMMENT 01-2005-081- 04-01-2021 ₂ SUB1-PLANS 3. POST PERMIT UPDATES 01-28-2022

9820 SE 35TH PLACE

ACHIN & MARY CHEN 9820 SE 35TH PLACE MERCER ISLAND, WA 98040

JOB ADDRESS:

PROJECT / CLIENT:

9820 SE 35TH PLACE MERCER ISLAND, WA 98040 PARCEL # 082405-9027

__20

RETAIN EXISTING HEADER

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 $-- \longrightarrow$

€X c

(EX) e

EXISTING STRUCTURE, NO STRUCTURALWORK

ELEVATIONS

Drawn By: JMG,RB Checked By: EB Owner Approval:

CONSTRUCTION DOCUMENTS

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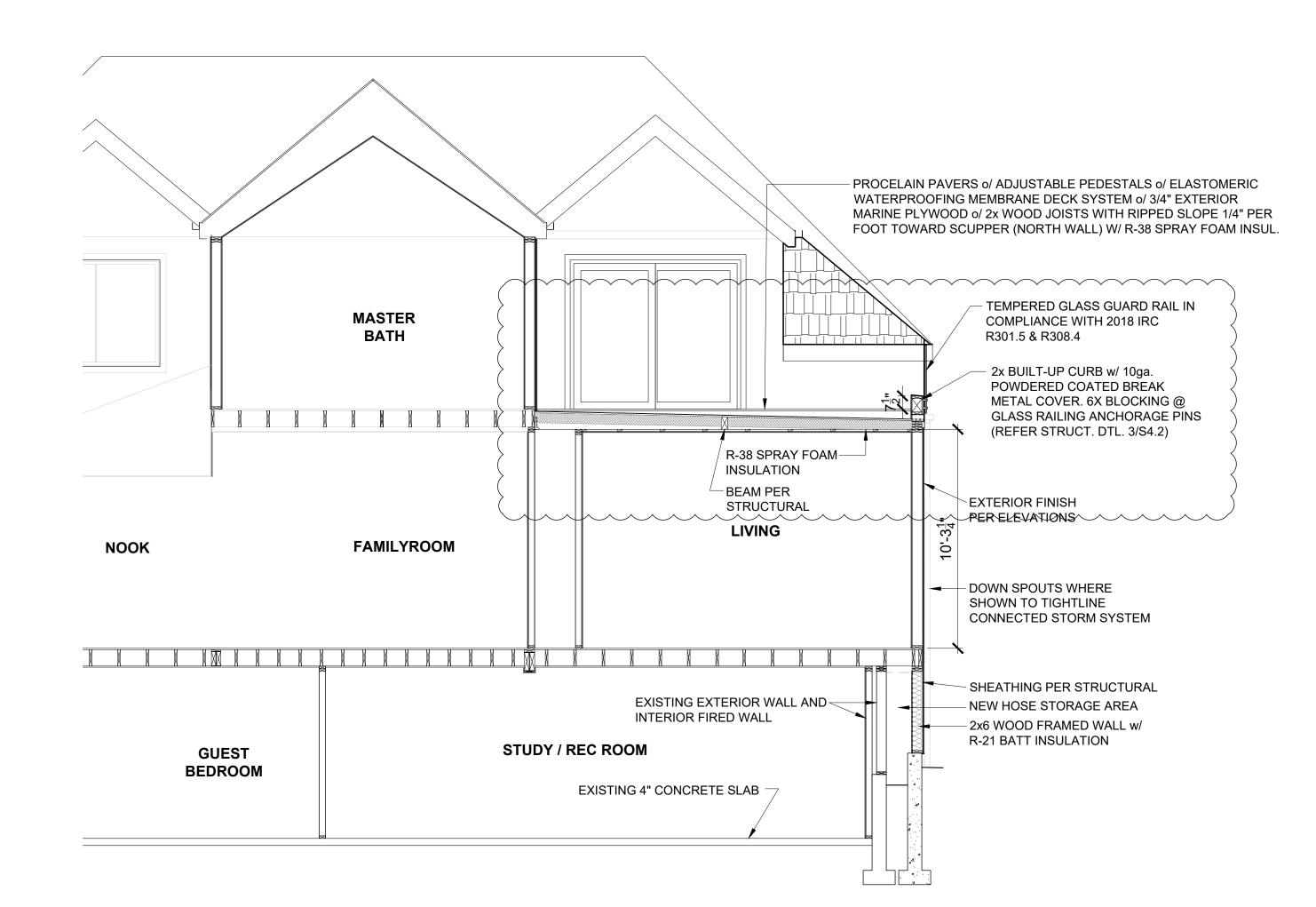
DATE: 12-22-2020

A4.0 PLOT SCALE: 1:1

THERMAL INSULATION:
Walls (below-grade, exterior):
Walls (below-grade, interior):
Walls (above-grade):
Headers:
Ceilings (advanced framing):
Ceilings (standard framing):
Ceilings (vaulted):
Floors:
Slab:
Solid doors:
Windows & doors with glazing:

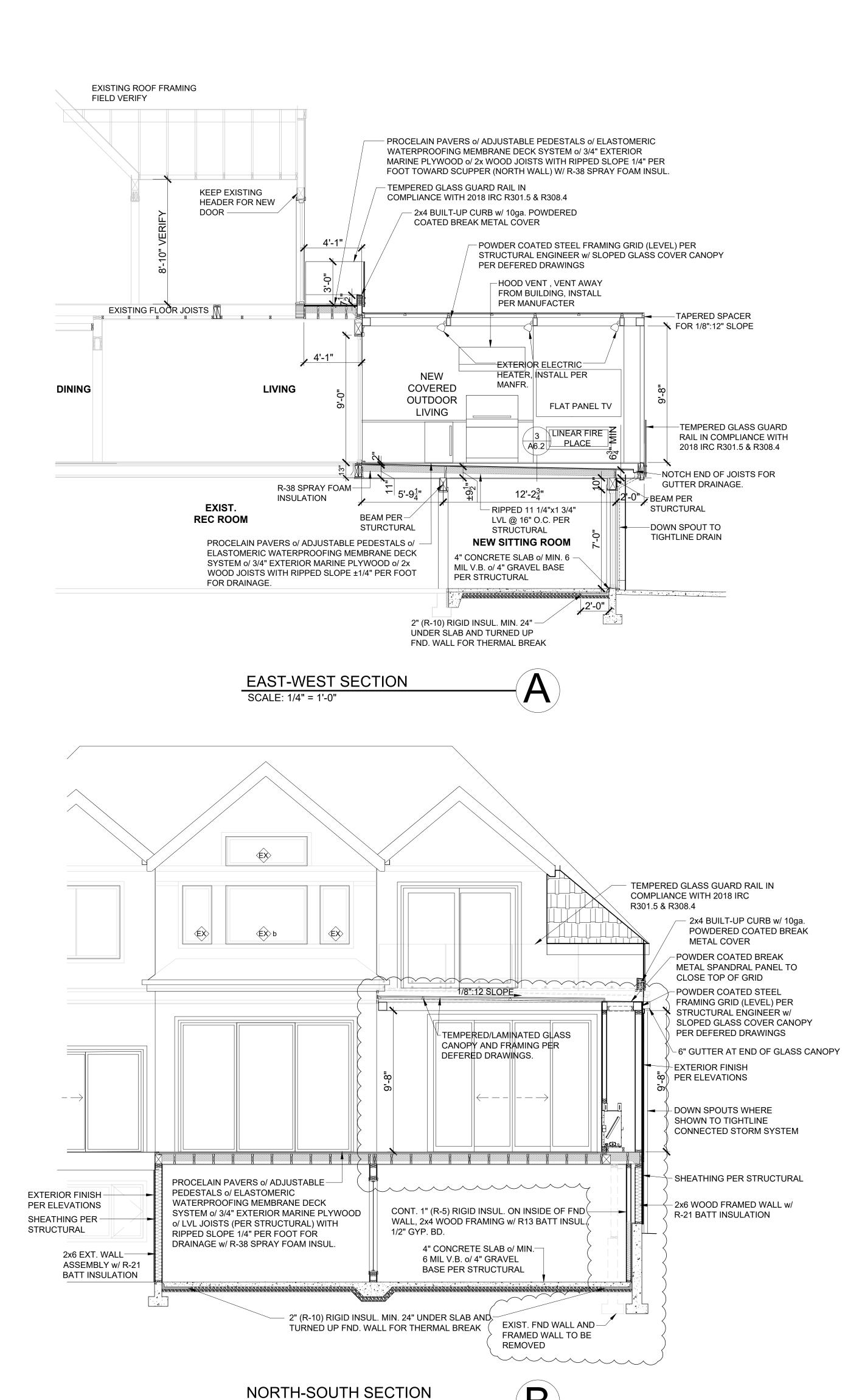
R-21 batt or rigid insulation
R-21 batt or rigid insulation
R-10 rigid insulation
R-38 batt
R-49 batt
Icynene with min R-49
R-30 batt or rigid insulation
R-10 water-resistant rigid insulation
U-value of .20 or better
U-value of .50 or better
U-value of .50 or better

R-10 rigid insulation

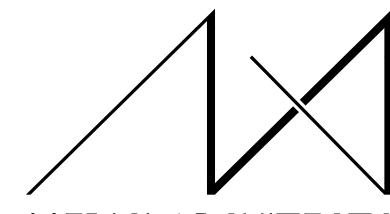


NORTH-SOUTH SECTION

SCALE: 1/4" = 1'-0" MSTR. BEDROOM DECK



SCALE: 1/4" = 1'-0"

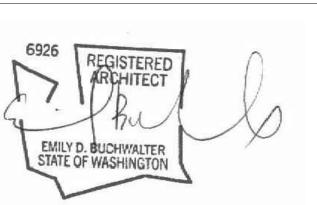


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INTAKE:	DATE:
REVISIONS:	DATE:
1./\damped POST PERMIT CHANGES	01-28-2022
2.	
3.	
4.	
5.	

PROJECT / CLIENT:

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PARCEL # 082405-9027

DRAWING NAME:

SECTIONS

Drawn By: JMG,RB
Checked By: EB
Owner Approval:

PHASE:

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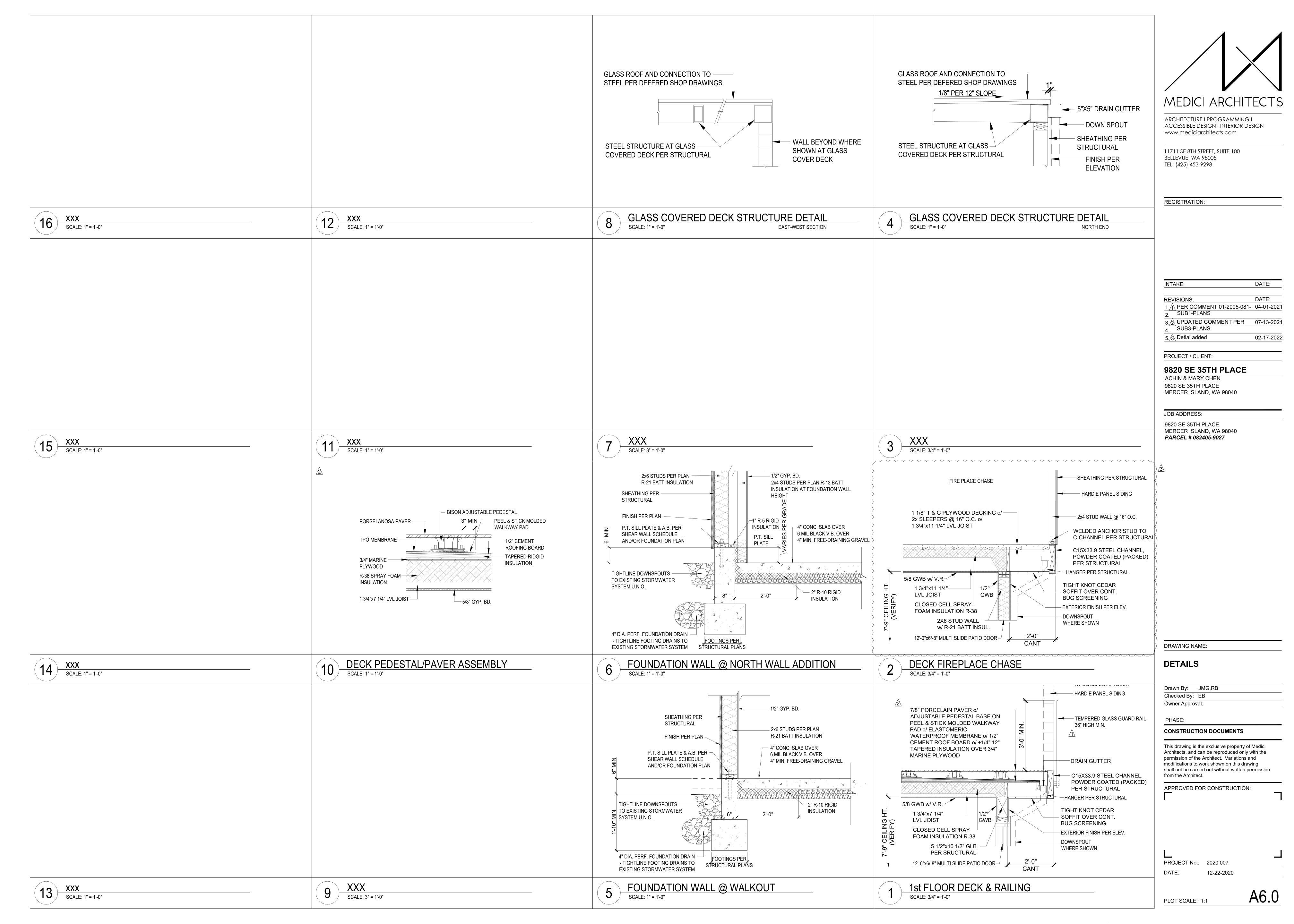
APPROVED FOR CONSTRUCTION:

PROJECT No.: 2020 007

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PLOT SCALE: 1:1

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SENERAL STRUCTURAL NOTES
(THE FOLLOWING APPLY UNLESS SHOWN OTHERWISE ON THE PLANS.)
  1. ALL MATERIALS, WORKMANSHIP, DESIGN, AND CONSTRUCTION FOR NEW CONSTRUCTION SHALL CONFORM TO THE DRAWINGS, SPECIFICATIONS, AND THE INTERNATIONAL BUILDING CODE
(IBC), 2015 EDITION.
  2. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL DRAWINGS FOR BIDDING AND CONSTRUCTION. CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS
FOR COMPATIBILITY AND SHALL NOTIFY ARCHITECT OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION. SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS AND DIMENSIONS OF DOOR AND
 WINDOW OPENINGS. SEE MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF MISCELLANEOUS MECHANICAL OPENINGS.
  3. CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS, MEMBER SIZES, AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON
THE DRAWINGS ARE INTENDED AS GUIDELINES ONLY AND MUST BE VERIFIED.
 4. DEMOLITION: CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS BEFORE COMMENCING ANY DEMOLITION. SHORING SHALL BE INSTALLED TO SUPPORT EXISTING CONSTRUCTION AS
REQUIRED AND IN A MANNER SUITABLE TO THE WORK SEQUENCE. EXISTING REINFORCING SHALL BE SAVED WHERE AND AS NOTED ON THE PLANS. SAW CUTTING, IF AND WHERE USED,
 SHALL NOT CUT EXISTING REINFORCING THAT IS TO BE SAVED. DEMOLITION DEBRIS SHALL NOT BE ALLOWED TO DAMAGE OR OVERLOAD THE EXISTING STRUCTURE. LIMIT CONSTRUCTION
 LOADING (INCLUDING DEMOLITION DEBRIS) ON EXISTING FLOOR SYSTEMS TO 40 PSF.
    4.1 ALL OPENINGS THROUGH EXISTING CONCRETE WALLS, SLABS AND BEAMS SHALL BE ACCOMPLISHED BY SAW CUTTING AND/OR CORING WHEREVER POSSIBLE. SAW CUT TO
 TERMINATE AT CORING AT CORNERS OF OPENING. DO NOT OVERCUT CORNERS.
    4.2 CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS AND LOCATION OF MEMBERS PRIOR TO CUTTING ANY OPENINGS.
    4.3 SMALL ROUND OPENINGS THROUGH CONCRETE SHALL BE ACCOMPLISHED BY CORE DRILLING IF POSSIBLE.
    4.4 WHERE NEW REINFORCING TERMINATES AT EXISTING CONCRETE, DOWEL BARS SHALL BE DRILLED AND EPOXIED INTO EXISTING CONCRETE TO MATCH NEW HORIZONTAL REINFORCING
 AS NOTED ON PLANS.
  5. CONTRACTOR SHALL CHECK FOR DRYROT AT ALL EXTERIOR WALLS. EXISTING TOILET ROOM FLOORS AND WALLS. AREAS SHOWING WATER STAINS, AND ALL WOOD MEMBERS IN THE
BASEMENT AND CRAWL SPACES. ALL ROT SHALL BE REMOVED AND DAMAGED MEMBERS SHALL BE REPLACED OR REPAIRED AS DIRECTED BY THE STRUCTURAL ENGINEER OR ARCHITECT.
  6. CONTRACTOR SHALL PROVIDE TEMPORARY BRACING FOR THE STRUCTURE AND STRUCTURAL COMPONENTS OF THE NEW CONSTRUCTION UNTIL ALL FINAL CONNECTIONS HAVE BEEN
COMPLETED IN ACCORDANCE WITH THE PLANS. THE CONTRACTOR SHALL ALSO PROVIDE TEMPORARY BRACING AND SHORING OF THE EXISTING BUILDING(S) IN WHICH PORTIONS OF THE
 EXISTING STRUCTURE ARE TO BE REMOVED OR MODIFIED. THIS TEMPORARY BRACING AND SHORING SHALL REMAIN IN PLACE UNTIL NEW CONSTRUCTION AND/OR STRUCTURAL MODIFICATIONS
ARE COMPLETED. THE CONTRACTOR SHALL DESIGN, PROVIDE MATERIALS FOR AND INSTALL (AND REMOVE IF NECESSARY) SUCH TEMPORARY WORK.
  7. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES REQUIRED TO PERFORM HIS WORK. STRUCTURAL
DESIGN OF THE BUILDING IS BASED ON RESISTANCE TO DEAD LOADS, CODE SPECIFIED LATERAL LOADS, AND MAXIMUM EXPECTED SERVICE LOADS. NO CONSIDERATION HAS BEEN GIVEN TO
LOADS WHICH WILL BE INDUCED BY ERECTION PROCEDURES. THE CONTRACTOR SHALL VERIFY, TO THE SATISFACTION OF HIM/HERSELF AND THE OWNER, THE ABILITY OF THE STRUCTURE TO
RESIST ALL ERECTION LOADS WITHOUT EXCEEDING THE ALLOWABLE STRESSES OF THE MATERIALS USED. WHERE ERECTION LOADS WOULD OVERSTRESS THE STRUCTURE, THE CONTRACTOR
SHALL SUBMIT DESIGN DOCUMENTS FOR TEMPORARY BRACING AND STRENGTHENING, INCLUDING FABRICATION AND ERECTION DRAWINGS, TO THE ARCHITECT FOR REVIEW. THESE DOCUMENTS
SHALL BEAR THE SEAL AND SIGNATURE OF A REGISTERED STRUCTURAL ENGINEER IN THE STATE OF WASHINGTON. THE CONTRACTOR SHALL PROVIDE, INSTALL AND IF NECESSARY REMOVE
SUCH TEMPORARY WORK AS REQUIRED.
  8. CONTRACTOR-INITIATED CHANGES SHALL BE SUBMITTED IN WRITING TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO FABRICATION OR CONSTRUCTION.
 CHANGES SHOWN ON SHOP DRAWINGS ONLY WILL NOT SATISFY THIS REQUIREMENT.
  9. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED, BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN,
 SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE STRUCTURAL ENGINEER.
  10. ALL STRUCTURAL SYSTEMS WHICH ARE TO BE COMPOSED OF COMPONENTS TO BE FIELD ERECTED SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY,
 HANDLING, STORAGE AND ERECTION IN ACCORDANCE WITH INSTRUCTIONS PREPARED BY THE SUPPLIER.
  11. INSPECTIONS: INSPECTIONS OF THE WOOD FRAMING, THE STEEL REBAR AND WOOD FORMS FOR CONCRETE FOOTINGS & FOUNDATIONS, AND CONCRETE SLABS ARE REQUIRED PER IBC
SECTION 110.3.
 12. SPECIAL INSPECTION: SHALL BE PERFORMED BY A CERTIFIED TESTING AGENCY, DESIGNATED BY THE ARCHITECT OR ENGINEER, AND APPROVED BY THE OWNER.
THE SPECIAL INSPECTION AGENCIES SHALL PRODUCE REPORTS AND KEEP RECORDS PER IBC SECTION 1704.2.4.
 THE SPECIAL INSPECTION AGENCIES SHALL INSPECT FABRICATORS PER IBC SECTION 1704.2.5. IF FABRICATOR IS REGISTERED & APPROVED TO PERFORM THE WORK WITHOUT SPECIAL
INSPECTION PER IBC 1704.2.5.1, FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL PER IBC SECTION 1704.2.5.1. AT COMPLETION OF FABRICATION.
 THE SPECIAL INSPECTION AGENCIES DUTIES SHALL INCLUDE THE FOLLOWING:
    12.1 STEEL CONSTRUCTION:
VERIFY AND/OR INSPECT STEEL CONSTRUCTION PER QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360. THIS INCLUDES (BUT IS NOT LIMITED TO) STRUCTURAL STEEL,
HIGH-STRENGTH BOLTING, WELDING, AND JOINTS OF STEEL FRAMES.
    12.2 PILE & PIER FOUNDATIONS:
( SPECIAL INSPECTIONS OF PIER OR PILE FOUNDATIONS ARE REQUIRED PER IBC SECTIONS 1705.7 AND 1704.8.
13. SHOP DRAWINGS FOR REINFORCING STEEL STRUCTURAL STEEL, GLUED LAMINATED MEMBERS, ENGINEERED LUMBER SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER
FOR REVIEW PRIOR TO FABRICATION OF THESE ITEMS.
ENGINEER OF RECORD SHALL REVIEW SHOP DRAWINGS FOR DESIGN INTENT ONLY. DIMENSIONS AND QUANTITIES ARE NOT GUARANTEED BY THE ENGINEER OF RECORD. AND THEREFORE. MUST
BE VERIFIED BY THE GENERAL CONTRACTOR. DRAWINGS FOR COMPONENTS DESIGNED PRIMARILY BY OTHERS SHALL BE APPROVED BY THE COMPONENT DESIGNER PRIOR TO CURSORY REVIEW
BY THE ENGINEER OF RECORD FOR LOADS IMPOSED ON THE BASIC STRUCTURE. SUBMITTALS SHALL INCLUDE A REPRODUCIBLE AND A COPY; REPRODUCIBLE WILL BE REVIEWED AND
RETURNED. SHOP DRAWINGS MUST BE REVIEWED AND STAMPED BY CONTRACTOR PRIOR TO REVIEW BY ENGINEER.
  14. PRE-MANUFACTURED, PRE-ENGINEERED STRUCTURAL COMPONENTS SHALL BE DESIGNED BASED ON THE CRITERIA PRESENTED IN THE CONTRACT DOCUMENTS. THE COMPONENT
DESIGNER IS RESPONSIBLE FOR CODE CONFORMANCE, TEMPORARY AND PERMANENT BRACING AND ALL NECESSARY CONNECTIONS, INCLUDING CONNECTIONS TO THE PRIMARY STRUCTURE, NOT
 SPECIFICALLY CALLED OUT ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS. SHOP DRAWINGS SHALL INDICATE THE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON THE PRIMARY
STRUCTURE. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED PER PARAGRAPH "A.13." OF THESE NOTES.
  15. DEFERRED SUBMITTALS — THE FOLLOWING ITEMS ARE CONSIDERED TO BE DEFERRED SUBMITTALS UNDER SECTION 107.3.4.1 OF THE INTERNATIONAL BUILDING CODE AND MUST BE
 SUBMITTED TO THE ARCHITECT OR THE ENGINEER FOR REVIEW. THESE ITEMS WILL THEN BE FORWARDED TO THE BUILDING OFFICIAL FOR APPROVAL. THE DEFERRED SUBMITTAL ITEMS SHALL
NOT BE INSTALLED UNTIL THEIR DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL. DESIGN SUBMITTALS SHALL BEAR THE STAMP AND SIGNATURE OF A
REGISTERED STRUCTURAL ENGINEER IN THE STATE OF WASHINGTON
     * PRE-ENGINEERED GLASS STAIR TREADS, GLASS GUARDRAILS AND GLASS ROOF.
 B. DESIGN CRITERIA
  1. DESIGN LOADS
     ROOF LIVE LOAD
                                           25 PSF (SNOW*, IS=1.0)
     ROOF DEAD LOAD
                                           15 PSF (20 PSF @ GLASS ROOF)
     FLOOR LIVE LOAD (RESIDENTIAL)
                                           40 PSF (REDUCIBLE)
     FLOOR DEAD LOAD
                                           15 PSF
                                           60 PSF (REDUCIBLE)
     DECK LIVE LOAD
     DECK DEAD LOAD
                                           20 PSF
      WIND (ASCE 7-10)
                                 V_{\text{LILT}} = 110 \text{ MPH}, V_{\text{ASD}} = 85 \text{ MPH}, (3 \text{ SEC GUST})
                                 ENCLOSED BUILDING, EXPOSURE "C", IW=1.0, KZT = 1.0
     EARTHQUAKE (ASCE 7-10)
                                 SITE CLASS D
                                 OCCUPANCY CATEGORY II (IE = 1.0)
                                 SEISMIC DESIGN CATEGORY D
                                 SS = 1.382G, S1 = 0.531G
                                 SDS=0.921G, SD1 = 0.531G
                                 R=6.5, R=1.3
                                 V_{IJIT} = C_S W = 0.129W
                                 EQUIVALENT LATERAL FORCE PROCEDURE
                                 LATERAL LOADS ARE RESISTED BY STRUCTURAL WOOD
                                 PANEL SHEAR WALLS & DIAPHRAGMS
     ALLOWABLE SOIL PRESSURE** ...... 1,500 PSF
     55 PCF AT-REST\14H SEISMIC
                                             250 PSF PASSIVE
                                             0.35 COEFFICIENT OF FRICITION
*FOR-SNOW-DRIFT-GALCULATIONS, PG=15-PSF
     **SOILS REPORT REFERENCE: GEOTECHNICAL REPORT. PROPOSED ADDITION AND NEW GARAGE, 9820 SE 35<sup>th</sup> PL, MERCER ISLAND, WA, PREPARED BY GEO GROUP NORTHWEST INC,
DATED AUGUST 20, 2020, REPORT #G-5207. GEO GROUP NORTHWEST DAILY FIELD REPORT DATED 02/08/2022.
  1. FOUNDATION EXCAVATION, BACKFILL AND COMPACTION SHALL CONFORM TO SPECIFICATION REQUIREMENTS. THIS CONSTRUCTION WORK, INCLUDING DRAINAGE, SHORING AND SUCH OTHER
RELATED WORK AS REQUIRED, SHALL BE CONDUCTED BY THE CONTRACTOR UNDER THE OBSERVATION AND DIRECTION OF THE GEOTECHNICAL ENGINEER.
  2. FOOTINGS SHALL BEAR ON SOLID UNDISTURBED EARTH (CONTROLLED, COMPACTED STRUCTURAL FILL OR BOTH) AT LEAST 18" BELOW LOWEST ADJACENT FINISHED GRADE. MATERIAL TO
BE COMPACTED TO 95% MINIMUM OF MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D1557.
 3. FOOTINGS MAY BE POURED IN NEAT EXCAVATIONS PROVIDED SIZE IS INCREASED 3" AT EACH INTERFACE WITH SOIL.
 4. ALL FOOTING EXCAVATIONS SHALL BE HAND CLEANED PRIOR TO PLACING CONCRETE.
 5. ALL ABANDONED FOOTINGS, UTILITIES, ETC. THAT INTERFERES WITH NEW CONSTRUCTION SHALL BE REMOVED.
 6. CONTRACTOR SHALL PROVIDE FOR DESIGN AND INSTALLATION OF ALL CRIBBING, SHEATHING, AND SHORING REQUIRED TO SAFELY RETAIN EXCAVATIONS.
  7. BACKFILL BEHIND ALL WALLS WITH WELL DRAINING, GRANULAR FILL MATERIAL, AND PROVIDE PERFORATED PIPE DRAINS AS DESCRIBED IN THE SOILS REPORT. BACKFILL BEHIND WALLS
```

SHALL NOT BE PLACED BEFORE THE WALL IS PROPERLY SUPPORTED BY THE FLOOR SLAB, OR TEMPORARY BRACING. ALL FOOTINGS SHALL BE CENTERED BELOW CENTERLINE OF COLUMNS

OR WALLS ABOVE. UNLESS NOTED OTHERWISE.

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PIN PILE FOUNDATION
   1. PIN PILE TO BE 2" DIAMETER SCHEDULE GALVANIZED 80 PIPE. PIN PILE DESIGN CAPACITY IS 6 KIPS
  2. PIN PILES ARE TO BE DRIVEN USING A 90-POUND OR 140-POUND PNEUMATIC JACKHAMMER UNTIL REFUSAL. REFUSAL IS DEFINED AS A PENETRATION RESISTANCE OF LESS THAN 1
/INCH OF PENETRATION PER MINUTE OF SUSTAINED DRIVING.
  3. PIN PILE TO EXTEND 6" MINIMUM INTO PILE CAP. PROVIDE COMPRESSION FIT CAP.
  4. PIPE PILE TO BE SPLICED WITH COMPRESSION FIT SLEEVED COUPLER OR COMPLETE PENETRATION WELD.
  5. THE GEOTECHNICAL ENGINEER OF RECORD OR HIS/HER REPRESENTATIVE SHALL PROVIDE FULL—TIME OBSERVATION OF PILE INSTALLATION AND TESTING TO VERIFY THE DRIVING REFUSAL
> 6. A MINIMUM OF 3% OF THE PILES (1 MINIMUM AND UP TO 5 PILES MAXIMUM) SHOULD BE LOAD TESTED TO VERIFY DESIGN LOAD CAPACITIES. ALL LOAD TESTS SHALL BE PERFORMED
(IN ACCORDANCE WITH THE PROCEDURE OUTLINED IN ASTM D1143. THE MAXIMUM TEST LOAD SHALL BE 2X THE THE DESIGN LOAD. TEST LOAD = 12 KIPS FOR 2 INCH PILE.
  1. ULTIMATE STRENGTH DESIGN PER INTERNATIONAL BUILDING CODE AND ACT 318–14.
 2. CONCRETE SHALL MEET THE FOLLOWING MINIMUM REQUIREMENTS
     2.1 CONCRETE SHALL ATTAIN A 28-DAY STRENGTH OF F'C = 3,500 PSI AND MIX SHALL CONTAIN NOT LESS THAN 5-1/2 SACKS OF CEMENT PER CUBIC YARD AND SHALL BE
PROPORTIONED TO PRODUCE A SLUMP OF 5" OR LESS. EXPOSURE CLASS F1, S0, WO & CO. DESIGN IS BASED ON F'C = 2,500 PSI.
 3. THE MINIMUM AMOUNTS OF CEMENT AND MAXIMUM AMOUNTS OF WATER MAY BE CHANGED IF A CONCRETE DESIGN MIX IS SUBMITTED TO THE STRUCTURAL ENGINEER AND THE BUILDING
DEPARTMENT FOR APPROVAL TWO WEEKS PRIOR TO PLACING ANY CONCRETE. THE CONCRETE PERFORMANCE MIX SHALL INCLUDE THE AMOUNTS OF CEMENT. FINE AND COARSE AGGREGATE.
WATER AND ADMIXTURES AS WELL AS THE WATER CEMENT RATIO, SLUMP, CONCRETE YIELD AND SUBSTANTIATING STRENGTH DATA IN ACCORDANCE WITH ACI 318, CHAPTERS 19 AND 26.
      ALL CONCRETE EXPOSED TO FREEZING TEMPERATURES WHILE CURING AND ALL CONCRETE PERMANENTLY EXPOSED TO WEATHER SHALL BE AIR-ENTRAINED WITH AN AIR-ENTRAINING
AGENT CONFORMING TO ACI 318. TOTAL AIR CONTENT SHALL BE 6% IN ACCORDANCE WITH TABLE 19.3.3.1.
     NO ADMIXTURES, OTHER THAN FOR AIR-ENTRAINMENT AS NOTED ABOVE, SHALL BE USED WITHOUT PRIOR REVIEW BY THE STRUCTURAL ENGINEER.
  4. REINFORCING
      REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615 (INCLUDING SUPPLEMENT S1), GRADE 60, FY = 60,000 PSI. REINFORCEMENT FOR COLUMNS, WALLS, WALL
TO FOOTING DOWELS, AND WOOD SHEAR WALL HOLD DOWNS TO BE A706 UNLESS CERTIFIED MILL CERTIFICATES CONFORMING TO ACI 318 20.2.2.5 ARE PROVIDED.
      WELDED WIRE REINFORCEMENT: ASTM A82 AND ASTM A185, SPLICE WITH AT LEAST ONE FULL MESH. PLACE AT MID-DEPTH, OR SLIGHTLY ABOVE, OF SLAB. MATERIAL TO BE
SUPPLIED IN FLAT SHEETS.
 5. REINFORCING STEEL SHALL BE DETAILED (INCLUDING HOOKS AND BENDS) IN ACCORDANCE WITH ACI 318 (LATEST EDITION). LAP ALL CONTINUOUS REINFORCEMENT PER NOTE D.6.
PROVIDE CORNER BARS AT ALL WALL INTERSECTIONS. LAP CORNER BARS PER NOTE D.6. LAP ADJACENT MATS OF WELDED WIRE REINFORCEMENT A MINIMUM OF 8" AT SIDES AND ENDS.
     NO BARS PARTIALLY EMBEDDED IN HARDENED CONCRETE SHALL BE FIELD BENT UNLESS SPECIFICALLY SO DETAILED OR APPROVED BY THE STRUCTURAL ENGINEER.
  6. REINFORCING STEEL LAPS AND EMBEDMENT SHALL BE AS NOTED BELOW, UNLESS NOTED OTHERWISE:
      DEVELOPMENT LENGTH - COMPRESSION 20 BAR DIAM. - 24" MINIMUM
      DEVELOPMENT LENGTH — TENSION
                                            48 BAR DIAM. — (#11 BAR — 54 BAR DIA.)
      DEVELOPMENT LENGTH - TENSION, TOP BAR* 64 BAR DIAM. - (#11 BAR - 70 BAR DIA.)
      LAP SPLICE LENGTH - COMPRESSION
                                            30 BAR DIAM. — 24" MINIMUM
      LAP SPLICE LENGTH - TENSION
                                            64 BAR DIAM. — (#11 BAR — 70 BAR DIAM.)
     LAP SPLICE LENGTH - TENSION, TOP BAR* 80 BAR DIAM. - (#11 BAR - 90 BAR DIAM.)
    *TOP BARS ARE HORIZONTAL REINFORCEMENT SO PLACED THAT MORE THAN 12" OF CONCRETE IS CAST IN THE MEMBER BELOW THE BAR.
    ALL HOOKS SHALL BE "STANDARD" IN ACCORDANCE WITH ACI 318. REINFORCING SHALL NOT BE TACK WELDED. DO NOT WELD GRADE 60 REINFORCING.
  7. HIGH STRENGTH THREADED RODS (STRESSED AND NON-STRESSED) SHALL BE DYWIDAG THREADBARS WITH APPROPRIATE ANCHORAGE PLATES, NUTS, AND COUPLERS AS MANUFACTURED
BY DICKERHOFF AND WINDMANN, INC., IN CONFORMANCE WITH ASTM A722 (FPU = 150,000).
  8. MECHANICAL SPLICING OF REINFORCING BARS, WHERE INDICATED ON THE DRAWINGS, SHALL BE BY AN INTERNATIONAL CODE COUNSEL (ICC) APPROVED SYSTEM (SUCH AS LENTON,
FOX-HOWLETT. ETC.) AND SHALL DEVELOP 125% OF THE SPECIFIED YIELD STRENGTH OF THE BARS. SPLICE LOCATIONS OF ALTERNATE BARS SHALL BE OFFSET BY A DISTANCE WHICH
CONFORMS TO THE ICC REPORT OF THE SPLICE USED.
  9. CONCRETE PROTECTION (COVER) FOR REINFORCING STEEL SHALL BE AS FOLLOWS:
      FOOTINGS AND OTHER UNFORMED SURFACES. EARTH FACE
     FORMED SURFACES EXPOSED TO EARTH (I.E. WALLS BELOW GROUND) OR WEATHER
                                (#6 BARS OR LARGER)
                                                          1-1/2"
                                (#5 BARS OR SMALLER)
      COLUMN TIES OR SPIRALS AND BEAM STIRRUPS
                                                           1-1/2"
      JOISTS, SLABS AND WALLS (INTERIOR FACE)
  10. NON-SHRINK GROUT SHALL BE FURNISHED BY AN APPROVED MANUFACTURER AND SHALL BE MIXED AND PLACED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED
RECOMMENDATIONS. GROUT SHALL BE NON-SHRINK, CEMENT-BASED AND HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF F'C = 5000 PSI WHEN TESTED IN ACCORDANCE WITH
ASTM C109.
 11. ADHESIVE ANCHOR SYSTEM SHALL BE SET-XP OR SET-3G EPOXY BY SIMPSON STRONG-TIE, HIT-HY 200-A OR HIT-RE 500 V3 BY HILTI, AC200+ BY DEWALT, OR APPROVED EQUAL.
 12. ALL WOOD PLATES IN DIRECT CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE—TREATED (SEE WOOD SECTION).
E. STRUCTURAL STEEL
 1. STRUCTURAL STEEL DESIGN, FABRICATION, AND ERECTION SHALL BE BASED ON THE AISC 360 "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL
FOR BUILDINGS," LATEST EDITION, PLUS ALL REFERENCED CODES.
 2. ALL "W" (WIDE FLANGE BEAM AND COLUMN) SHAPES SHALL CONFORM TO ASTM A992. HP SHAPES SHALL CONFORM TO ASTM A572, FY = 50 KSI. PLATES, BARS AND OTHER
ROLLED SHAPES SHALL CONFORM TO ASTM A36, FY = 36 KSI, UNLESS CALLED OUT OTHERWISE ON PLAN. STEEL PIPE SHALL BE SCHEDULE 40 CONFORMING TO ASTM A53, TYPE E OR S,
GRADE B, FY = 35 KSI. RECTANGULAR HSS SHALL CONFORM TO ASTM A500, GRADE B, FY = 46 KSI, ROUND HSS SHALL CONFORM TO ASTM A500, GRADE B, FY = 42 KSI.
 3. ANCHOR BOLTS SHALL CONFORM TO ASTM A307 AND HAVE A WELDED HEAD. EMBED ANCHOR BOLTS A MINIMUM OF 7" INTO CONCRETE.
 5. ALL CONNECTION BOLTS AT STEEL/STEEL CONNECTIONS SHALL BE ASTM A325 OR ASTM A490 AND SHALL BE INSTALLED, TIGHTENED, AND INSPECTED IN ACCORDANCE WITH THE
AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A 490 BOLTS." THE CRITERIA FOR SNUG-TIGHT CONNECTIONS SHALL APPLY TO ALL CONNECTIONS UNLESS SPECIFICALLY
NOTED AS SLIP-CRITICAL ON THE STRUCTURAL DRAWINGS. WHERE CONNECTIONS ARE NOTED AS SLIP-CRITICAL, THE CONTRACTOR SHALL INSTALL PER CRITERIA FOR SLIP-CRITICAL
CONNECTIONS. SLIP-CRITICAL CONNECTIONS SHALL USE LOAD INDICATOR WASHERS OR TENSION CONTROL BOLTS. ALL BOLT HOLES SHALL BE STANDARD SIZE, UNLESS NOTED OTHERWISE.
 5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE COORDINATION OF THE SELECTION OF OPTIONAL DETAILS SHOWN ON THE DRAWINGS. THE CONTRACTOR SHALL ALSO BE
RESPONSIBLE FOR ALL ERECTION AIDS AND JOINT PREPARATIONS THAT INCLUDE, BUT ARE NOT LIMITED TO: ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS; WELDING PROCEDURES;
REQUIRED ROOT OPENINGS; ROOT FACE DIMENSIONS; GROOVE ANGLES; BACKING BARS; COPES; SURFACE ROUGHNESS VALUES; AND TAPERS OF UNEQUAL PARTS.
 6. EXPANSION BOLTS INTO CONCRETE SHALL BE "STRONG-BOLT 2" WEDGE ANCHORS BY SIMPSON STRONG-TIE, "KWIK BOLT TZ" WEDGE ANCHORS BY HILTI, POWER-STUD+ SD2, OR
APPROVED EQUAL INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED RECOMMENDATIONS, INCLUDING MINIMUM EMBEDMENT REQUIREMENTS. SPECIAL INSPECTION IS
REQUIRED FOR ALL EXPANSION BOLT AND INSERT INSTALLATION. SUBMIT MANUFACTURER'S DATA SHEETS AND ICC REPORTS FOR ENGINEER'S REVIEW.
 7. ALL WELDING SHALL BE IN CONFORMANCE WITH AISC AND AWS STANDARDS AND SHALL BE PERFORMED BY WABO CERTIFIED WELDERS USING E70XX ELECTRODES. WELDS, UNLESS
OTHERWISE NOTED, SHALL BE 3/16" CONTINUOUS FILLET WELDS. WELDS SHOWN ON DRAWINGS ARE MINIMUM SIZES. INCREASE WELD SIZE TO AWS MINIMUM SIZES, BASED ON PLATE
THICKNESS. WELDING OF REINFORCING BARS (IF REQUIRED) SHALL BE PERFORMED USING LOW HYDROGEN ELECTRODES. WELDING WITHIN 4" OF COLD BENDS IN REINFORCING STEEL IS NOT
PERMITTED. SEE REINFORCING NOTE FOR MATERIAL REQUIREMENTS OF WELDED BARS. WELDING PROCEDURES SHALL BE SUBMITTED TO THE OWNER'S TESTING AGENCY FOR REVIEW BEFORE
STARTING FABRICATION OR ERECTION.
     ALL WELDS SHALL BE VISUALLY INSPECTED AT THE SITE BY A QUALIFIED INSPECTOR.
     ALL COMPLETE PENETRATION WELDS SHALL BE ULTRASONICALLY TESTED AT THE PLANT OR SITE BY A QUALIFIED INSPECTOR.
     FIELD WELD ARROWS ARE SHOWN ONLY WHERE A FIELD WELD IS REQUIRED BY THE STRUCTURAL DESIGN. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING IF A WELD SHOULD BE
SHOP OR FIELD WELDED IN ORDER TO FACILITATE THE STRUCTURAL STEEL ERECTION.
F. CARPENTRY
 1. GLUED LAMINATED MEMBERS SHALL BE FABRICATED IN CONFORMANCE WITH ANSI STANDARD A190.1. EACH MEMBER SHALL BEAR AN AITC OR APA EWS IDENTIFICATION MARK AND
SHALL BE ACCOMPANIED BY AN AITC OR APA EWS CERTIFICATE OF CONFORMANCE. ALL SIMPLE SPAN BEAMS SHALL BE DOUGLAS FIR COMBINATION 24F-V4, FB = 2,400 PSI, FV = 240
PSI. ALL CANTILEVERED BEAMS SHALL BE DOUGLAS FIR COMBINATION 24F-V8, FB = 2400 PSI, FV = 240 PSI. CAMBER ALL GLULAM BEAMS TO 2,000' RADIUS, UNLESS SHOWN OTHERWISE
ON THE PLANS.
  2. FRAMING LUMBER SHALL BE GRADED AND MARKED IN CONFORMANCE WITH WCLIB STANDARD GRADING RULES FOR WEST COAST LUMBER, LATEST EDITION. FURNISH TO THE FOLLOWING
MINIMUM STANDARDS:
                                    SPECIES GRADE DESIGN STRESS
JOISTS AND RAFTERS:
                                                   FB = 850 PSI
                                   HEM.FIR #2
                                   HEM.FIR #2
                                                   FB = 850 PSI
BEAMS AND STRINGERS: 6X AND
                                  DOUG.FIR #1 FB = 1350 PSI
                     6X6, 6X8 DOUG.FIR #1
POSTS AND TIMBERS:
                                                   FC = 1000 PSI
                                                      FB = 1200 PSI
PLATES AT SHEAR
WALLS AND BEARING
STUDS, PLATES, &
                                    HEM.FIR \#2 FB = 850 PSI
MISC. LIGHT FRAMING:
     ALL LUMBER WITH A LEAST DIMENSION OF 2" (NOMINAL) SHALL BE STAMPED SURFACE-DRY AND SHALL HAVE A MOISTURE CONTENT WHEN SURFACED AND WHEN INSTALLED OF NOT
MORE THAN 19 PERCENT. LUMBER WITH A LEAST DIMENSION OF 4" (NOMINAL) OR GREATER SHALL BE STAMPED SURFACE-GREEN AND AIR-DRIED TO A MOISTURE CONTENT OF NOT MORE
THAN 19 PERCENT PRIOR TO ITS USE IN FRAMING THE STRUCTURE.
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3. MANUFACTURED LUMBER SHALL BE AS MANUFACTURED BY TRUS JOIST OR APPROVED EQUAL. REQUESTS FOR APPROVAL AS EQUAL WILL REQUIRE SUBMITTAL OF ICC REPORT
EQUIVALENT TO ESR-1387 FOR LAMINATED STRAND LUMBER (LSL), LAMINATED VENEER LUMBER (LVL), OR PARALLEL STRAND LUMBER (PSL). THE MINIMUM ALLOWABLE DESIGN VALUES ARE
AS FOLLOWS:
     LSL - FB = 2,250; FV = 400 PSI; E = 1,500,000 PSI
     LVL - FB = 2,600; FV = 285 PSI; E = 1,800,000 PSI
     PSL - FB = 2,900; FV = 290 PSI; E = 2,000,000 PSI
  4. SHEATHING SHALL BE APA PERFORMANCE RATED PANELS PER APA "PLYWOOD DESIGN SPECIFICATION", INCLUDING APPLICABLE SUPPLEMENTS, UNLESS NOTED OTHERWISE. PLYWOOD OR
 ORIENTED-STRAND BOARD (OSB) PANELS SHALL BE GRADE CD AND ALSO CONFORM TO DOC PS-1 & PS-2. ALL PANELS SHALL BE IDENTIFIED AS EXPOSURE 1 UNLESS NOTED OTHERWISE.
 PANEL RATING TO BE AS FOLLOWS UNLESS NOTED OTHERWISE:
          ROOF: 19/32'' (OR 5/8'') THICK, 40/20
                   15/32" THICK, 32/16, OR 1/2" THICK, 24/0
                   23/32" (OR 3/4") THICK, TONGUE & GROOVE, (48/24)
      UNLESS NOTED OTHERWISE ON THE PLANS, ROOF AND FLOOR SHEATHING SHALL BE LAID UP WITH GRAIN PERPENDICULAR TO SUPPORTS AND NAILED WITH 8D NAILS @ 6"OC TO
 FRAMED PANEL EDGES AND OVER STUD WALLS SHOWN ON PLANS AND @ 12"OC (10"OC AT FLOORS) TO INTERMEDIATE SUPPORTS. PROVIDE APPROVED SHEATHING EDGE CLIPS @ 16"OC AT
 UNBLOCKED ROOF SHEATHING EDGES. ALL FLOOR SHEATHING EDGES SHALL HAVE APPROVED TONGUE—AND—GROOVE JOINTS AND EDGE CLIPS. PROVIDE SOLID BLOCKING AT ALL EDGES ONLY
 WHERE NOTED ON PLANS. TOENAIL BLOCKING TO SUPPORTS WITH 16D NAILS, UNLESS NOTED OTHERWISE.
      UNLESS NOTED OTHERWISE ON THE PLANS, WALL SHEATHING MAY BE LAID UP HORIZONTALLY OR VERTICALLY, UNSUPPORTED EDGES SHALL BE BLOCKED AND ALL EDGES SHALL BE
 NAILED WITH 8D @ 6"OC, NAIL WITH 8D @ 12"OC AT INTERMEDIATE SUPPORTS. NAIL SHEAR WALL SHEATHING TO ALL HOLDOWN STUDS USING EDGE NAIL SPACING WHEN HOLDOWN STUD
DOES NOT OCCUR AT PANEL EDGES.
      SHEATHING NAILS SHALL BE DRIVEN FLUSH BUT SHALL NOT FRACTURE THE SURFACE OF THE SHEATHING.
 5. INTERIOR WOOD MEMBERS IN DIRECT CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE-TREATED WITH SODIUM BORATE (SBX). WOOD MEMBERS EXPOSED TO WEATHER
 (UNPAINTED) OR IN DIRECT CONTACT WITH SOIL SHALL BE PRESSURE-TREATED WITH ALKALINE COPPER QUATERNARY (ACQ). NOTE THAT ACQ IS EXTREMELY CORROSIVE TO METALS. SBX IS
 NONTOXIC TO THE ENVIRONMENT. PROVIDE TWO LAYERS OF ASPHALT IMPREGNATED BUILDING PAPER BETWEEN UNTREATED LEDGERS, BLOCKING, ETC., AND CONCRETE OR MASONRY. ALL
 METAL CONNECTORS IN CONTACT WITH "ACQ" PRESSURE—TREATED LUMBER OR FIRE—RETARDANT—TREATED LUMBER SHALL BE TYPE 304 OR 316 STAINLESS STEEL. THIS INCLUDES WASHERS,
SCREWS, NAILS, HANGERS, AND ANY OTHER MISCELLANEOUS LT. GAGE METAL CONNECTORS. WHERE ACQ LUMBER IS MISTAKENLY USED OR FOR FIRE—RETARDANT—TREATED LUMBER USED IN
 INTERIOR CONDITIONS, ASTM A 653, TYPE G185 ("HOT-DIP" GALVANIZED TO 1.85 OUNCES PER SQUARE FOOT) METAL CONNECTORS MAY BE USED IN LIEU OF STAINLESS STEEL. METAL
 CONNECTORS 1/2" THICK OR GREATER NEED NOT BE GALVANIZED FOR INTERIOR USE, NOR DO THEY NEED TO BE STAINLESS STEEL FOR EXTERIOR USE. METAL CONNECTORS 1/2" THICK
PLUS MUST BE GALVANIZED FOR EXTERIOR USE, UNLESS SPECIFIED OTHERWISE BY THE ARCHITECT.
 6. WOOD FASTENER NOTES - THE FOLLOWING APPLY UNLESS OTHERWISE SHOWN ON THE PLANS:
   6.1 NOTATIONS ON DRAWINGS RELATING TO FRAMING CLIPS, JOIST HANGERS AND OTHER CONNECTING DEVICES REFER TO CATALOG NUMBERS OF CONNECTORS MANUFACTURED BY THE
 SIMPSON STRONG—TIE COMPANY, DUBLIN, CALIFORNIA. EQUIVALENT DEVICES BY OTHER MANUFACTURERS MAY BE SUBSTITUTED, PROVIDED THEY HAVE ICC APPROVAL FOR EQUAL OR
 GREATER LOAD CAPACITIES. SUBMIT MANUFACTURER'S CATALOG AND ICC REPORTS TO ARCHITECT AND ENGINEER FOR REVIEW WHEN REQUESTING SUBSTITUTIONS. ALL SPECIFIED FASTENERS
 MUST BE USED AND PROPER INSTALLATION PROCEDURES MUST BE OBSERVED IN ORDER TO OBTAIN ICC APPROVED LOAD CAPACITIES. VERIFY THAT THE DIMENSIONS OF THE SUPPORTING
 MEMBER ARE SUFFICIENT TO RECEIVE THE SPECIFIED FASTENERS.
   6.2 NAILS SHALL BE MANUFACTURED IN CANADA OR THE UNITED STATES IN SIZES AND TYPES AS FOLLOWS, UNLESS NOTED OTHERWISE:
      PNEUMATIC NAILING — PLAIN SHANK, COATED OR GALVANIZED
        8D = .131 DIAMETER X 2-1/2" MINIMUM LENGTH
       10D = .148 DIAMETER X 3" MINIMUM LENGTH
        16D = .162 DIAMETER X 3-1/4" MINIMUM LENGTH
        20D = .192 DIAMETER X 4" MINIMUM LENGTH
      HAND NAILING - SINKERS, COATED
        8D = 11-1/2 \text{ GAGE X } 2-3/8
       10D = 11 \text{ GAGE X } 2-7/8"
        16D = 9 GAGE \times 3-1/4"
  7. WOOD FRAMING NOTES — THE FOLLOWING APPLY UNLESS OTHERWISE SHOWN ON THE PLANS:
 ARCHITECTURAL DRAWINGS.
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7.1 ALL WOOD FRAMING DETAILS NOT SHOWN OTHERWISE SHALL BE CONSTRUCTED TO THE MINIMUM STANDARDS OF THE INTERNATIONAL BUILDING CODE. MINIMUM NAILING, UNLESS OTHERWISE NOTED, SHALL CONFORM TO TABLE 2304.10.1 OF THE INTERNATIONAL BUILDING CODE. COORDINATE THE SIZE AND LOCATION OF ALL OPENINGS WITH MECHANICAL AND

7.2 WALL FRAMING: ALL STUD WALLS SHOWN AND NOT OTHERWISE NOTED SHALL BE 2X4 STUDS @ 16"OC AT INTERIOR WALLS AND 2X6 STUDS @ 16"OC AT EXTERIOR WALLS. TWO STUDS MINIMUM SHALL BE PROVIDED AT THE END OF ALL WALLS AND AT EACH SIDE OF ALL OPENINGS. UNLESS NOTED OTHERWISE A (2) 2X8 HEADER SHALL BE PROVIDED OVER ALL OPENINGS IN 2X4 STUD WALLS AND A (3) 2X8 HEADER OVER ALL OPENINGS IN 2X6 WALLS. SOLID BLOCKING FOR WOOD COLUMNS SHALL BE PROVIDED THROUGH FLOORS TO SUPPORT BELOW. PROVIDE CONTINUOUS SOLID BLOCKING AT MID-HEIGHT OF ALL STUD WALLS OVER 8'-0" IN HEIGHT.

ALL STUD WALLS SHOWN ON STRUCTURAL DRAWINGS SHALL HAVE THEIR LOWER PLATES ATTACHED TO WOOD FRAMING BELOW WITH 16D NAILS AT 12"OC STAGGERED OR BOLTED TO CONCRETE OR MASONRY WITH 5/8" DIAMETER BY 10" LONG ANCHOR BOLTS, EMBEDDED 7" AND SPACED AT 4'-0"OC MAXIMUM, UNLESS NOTED OTHERWISE (UNO) ON PLANS. SHEAR WALLS REQUIRE MINIMUM 3"X3"X1/4" SQUARE PLATE WASHERS AT ALL ANCHOR BOLTS. REFER TO THE STRUCTURAL PLANS AND SHEAR WALL SCHEDULE FOR REQUIRED SHEATHING AND NAILING.

7.3 FLOOR AND ROOF FRAMING: PROVIDE DOUBLE JOISTS UNDER ALL PARALLEL PARTITIONS AND AROUND ALL OPENINGS IN FLOORS OR ROOFS UNLESS OTHERWISE NOTED. PROVIDE BRIDGING @ 8'-0"OC AND SOLID BLOCKING AT ALL BEARING POINTS. COORDINATE THE SIZE AND LOCATION OF ALL OPENINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

TOENAIL JOISTS TO BEARING SUPPORTS WITH 16D NAILS. UNLESS NOTED OTHERWISE, ATTACH JOISTS TO FLUSH HEADERS OR BEAMS WITH SIMPSON "LU" SERIES METAL JOIST HANGERS TO SUIT JOIST SIZE. ALL DOUBLE JOISTS. BEAMS. AND SLOPED AND/OR SKEWED JOISTS SHALL BE CONNECTED TO FLUSH MEMBERS WITH U-SERIES JOIST HANGERS UNLESS NOTED OTHERWISE. SKEW AND SLOPE ALL CONNECTORS AS REQUIRED. FACE-NAIL ALL MULTI-JOIST BEAMS TOGETHER WITH 16D SPIKES @ 24"OC STAGGERED.





98 AC

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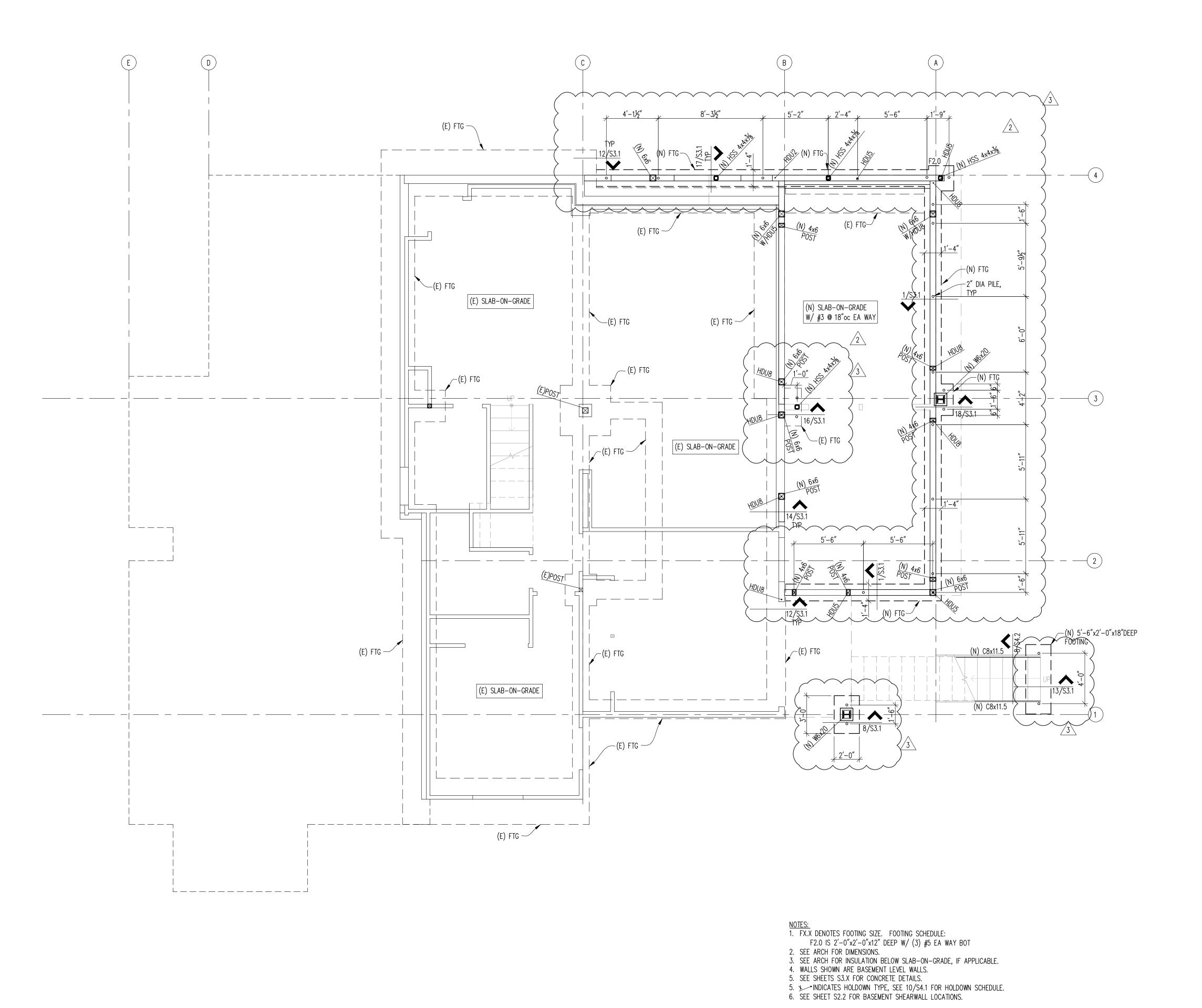
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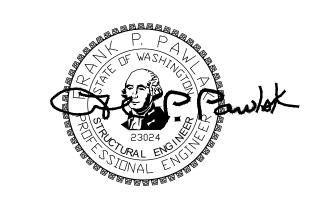
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GENERAL **STRUCTURAL** NOTES

SHEET TITLE



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NO. DATE

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3 2/18/22 REVISIONS

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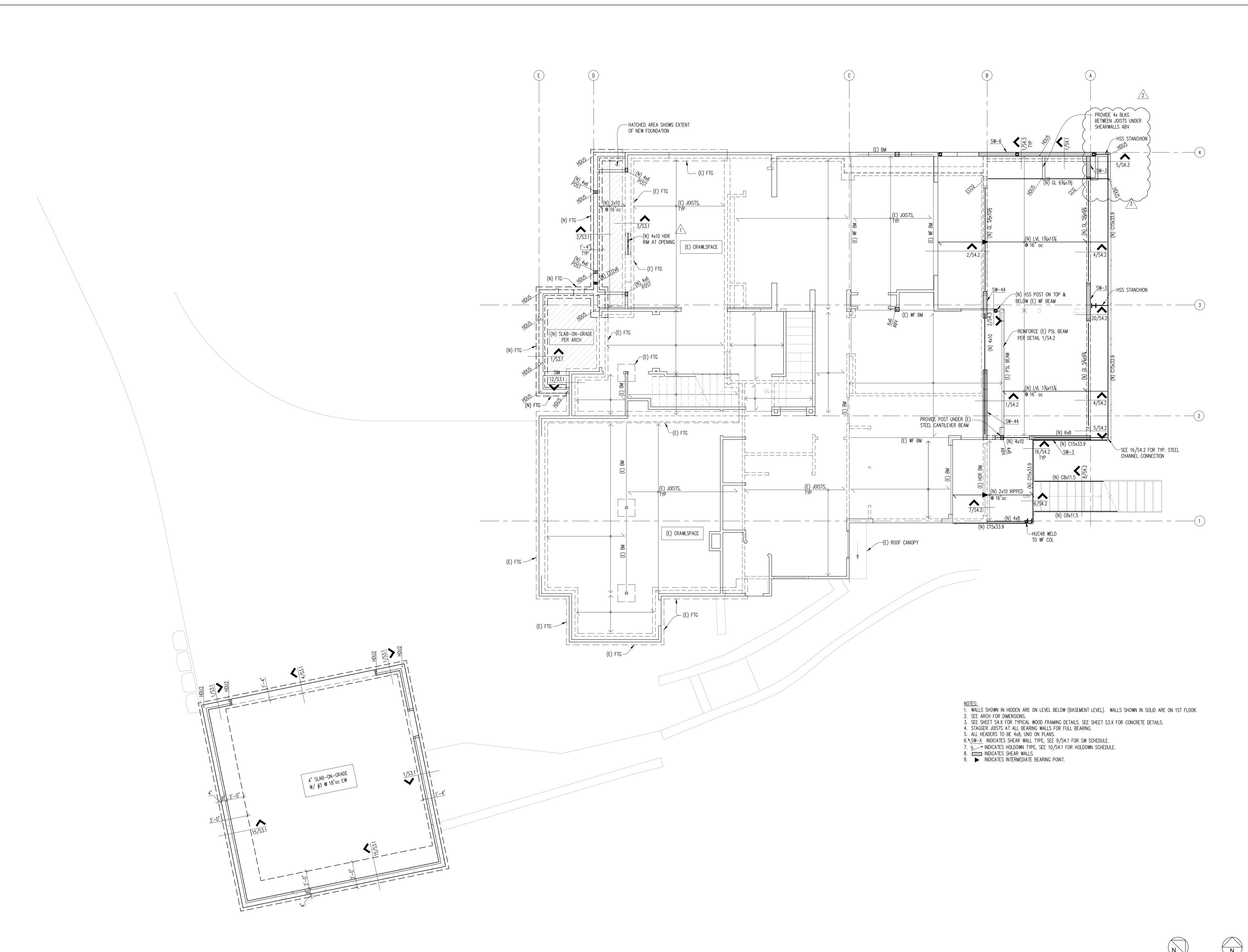
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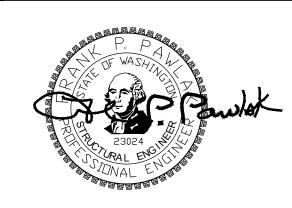
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S2.1

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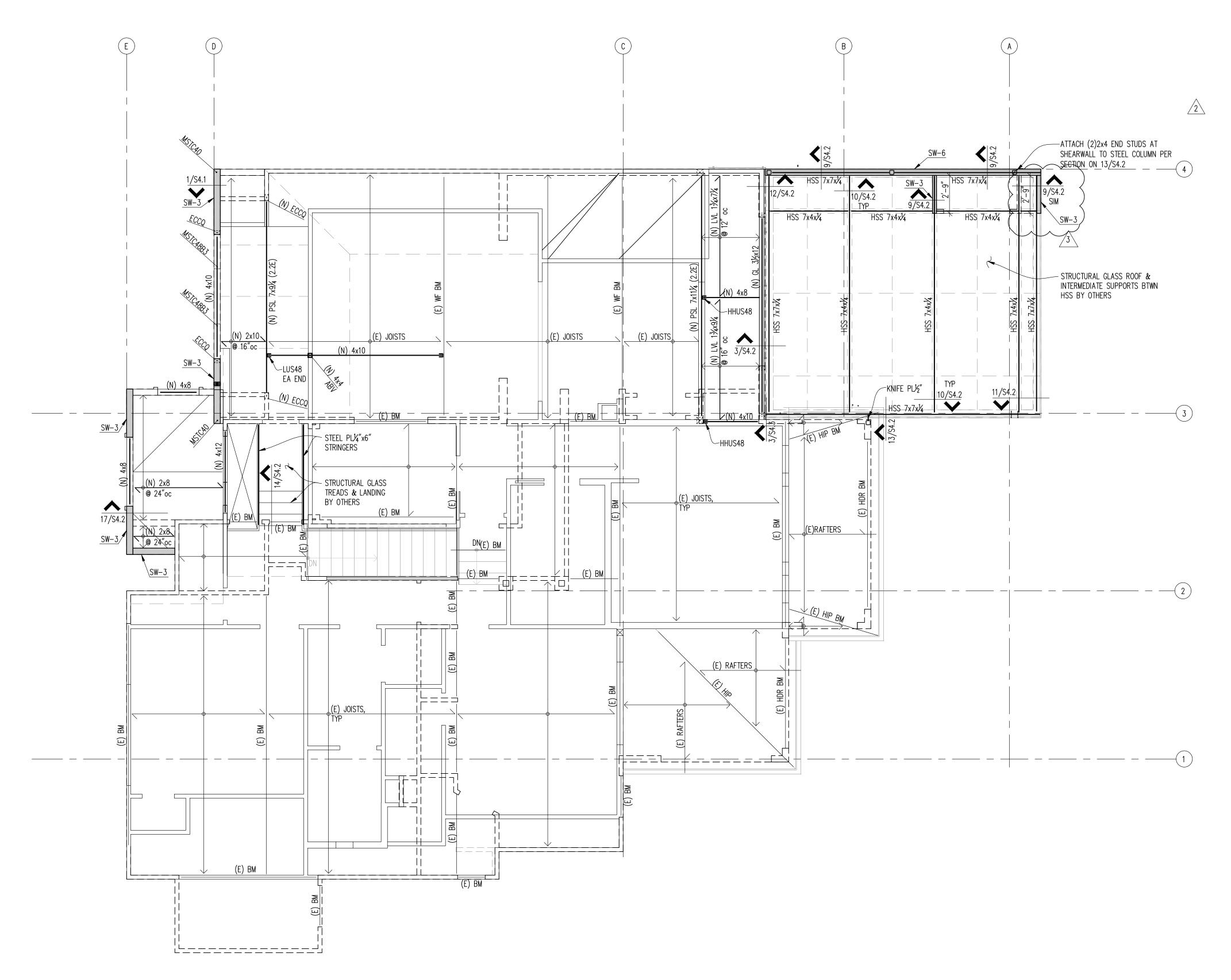
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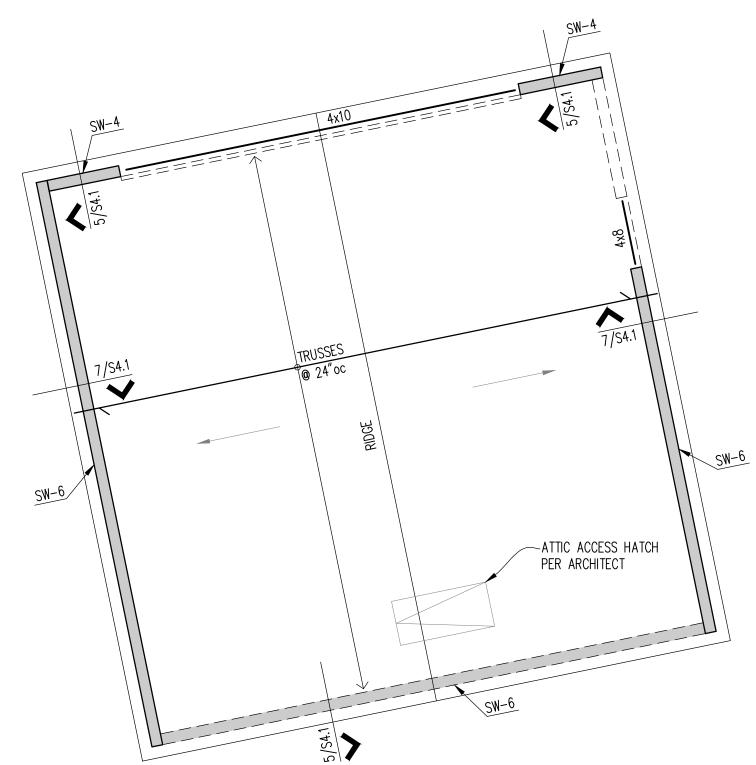
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1ST FLOOR FRAMING PLAN

S2.2





NOTES:

1. WALLS SHOWN IN HIDDEN ARE ON LEVEL BELOW (FIRST FLOOR). WALLS SHOWN IN SOLID ARE ON SECOND FLOOR.

2. SEE ARCH FOR DIMENSIONS.

3. SEE SHEET S4.X FOR TYPICAL WOOD FRAMING DETAILS.

4. STAGGER JOISTS AT ALL BEARING WALLS FOR FULL BEARING.

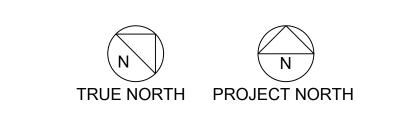
5. ALL HEADERS TO BE 4x8, UNO ON PLANS.

6. SW—X INDICATES SHEAR WALL TYPE, SEE 9/S4.1 FOR SW SCHEDULE.

7. INDICATES HOLDOWN TYPE, SEE 10/S4.1 FOR HOLDOWN SCHEDULE.

8. INDICATES SHEAR WALLS.

9. INDICATES INTERMEDIATE BEARING POINT.



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9820 SE 35TH PL

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2/18/22 REVISIONS

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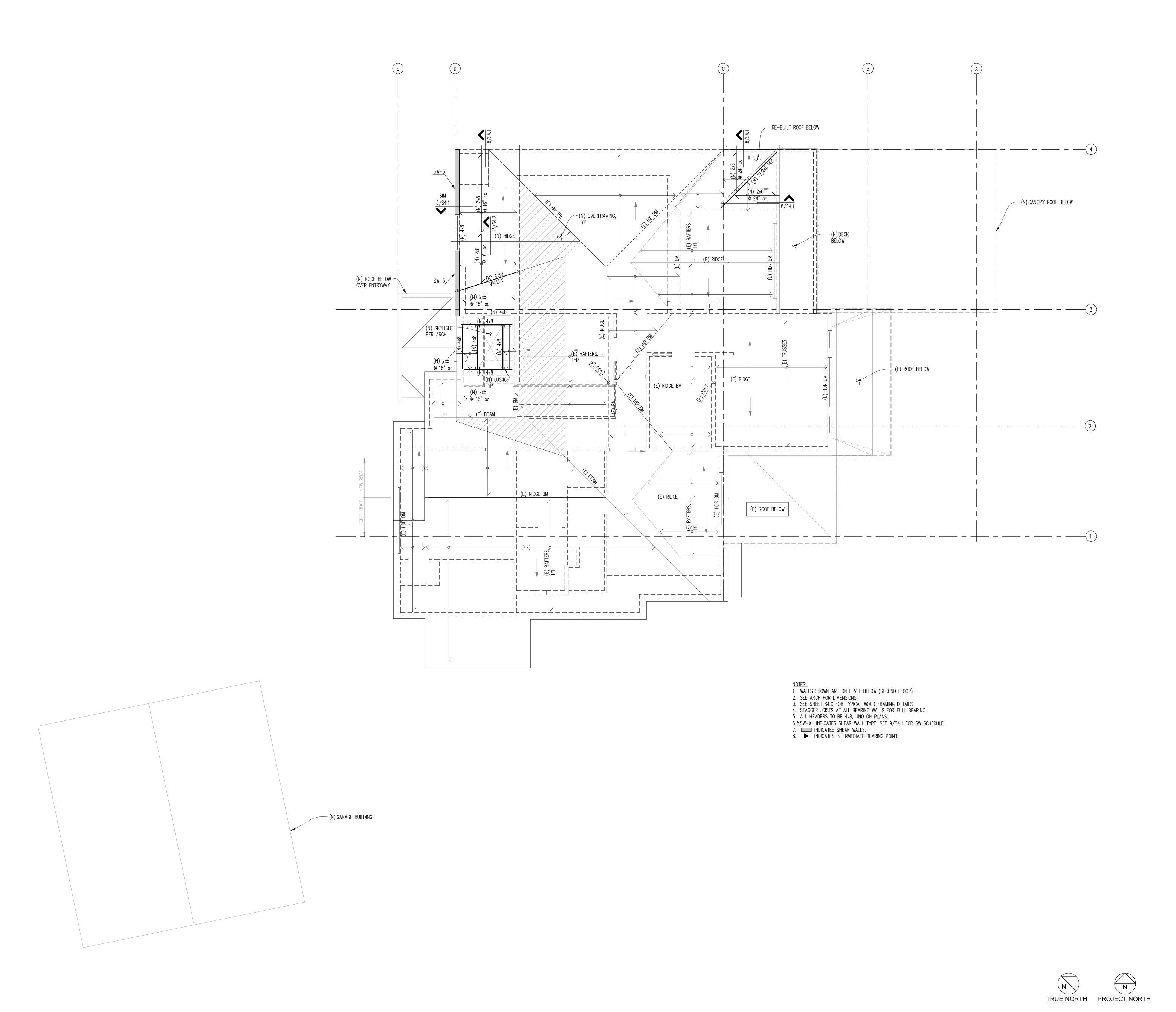
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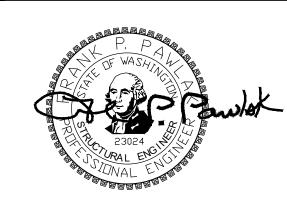
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2ND FLOOR FRAMING PLAN

S2.3







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 AS NOTED

 DRAWN
 SCALE

SDCI STAMP

SHEET TITLE

ROOF FRAMING PLAN

S2.4

ROOF FRAMING PLAN

1/4"=1'-0" 3

